Chapter 2

Free Vibration of Single Degree of Freedom Systems

2.1 \( \varepsilon_{st} = 5 \times 10^{-3} \, m \)

\[
\omega_n = \left(\frac{g}{\varepsilon_{st}}\right)^{\frac{1}{2}} = \left(\frac{9.81}{5 \times 10^{-3}}\right)^{\frac{1}{2}} = 44.2945 \, \text{rad/sec} = 7.0497 \, \text{Hz}
\]

2.2 \( \tau_n = 0.21 \, \text{sec} = 2\pi \sqrt{\frac{m}{k}} \), \( \sqrt{m} = 0.21 \sqrt{\frac{k}{2\pi}} \)

(i) \( \langle \tau_n \rangle_{\text{new}} = \frac{2\pi \sqrt{m}}{\sqrt{k_{\text{new}}}} = \frac{2\pi \sqrt{m}}{\sqrt{1.5k}} = \frac{2\pi}{\sqrt{1.5}} \left( \frac{0.21k}{2\pi} \right) = 0.1715 \, \text{sec} \)

(ii) \( \langle \tau_n \rangle_{\text{new}} = \frac{2\pi \sqrt{m}}{\sqrt{k_{\text{new}}}} = \frac{2\pi \sqrt{m}}{\sqrt{0.5k}} = 2\pi \left( \frac{0.21 \sqrt{k}}{2\pi} \right) \sqrt{0.5} = 0.2970 \, \text{sec} \)

2.3 \( \omega_n = 62.832 \, \text{rad/sec} = \sqrt{\frac{k}{m}} \), \( \sqrt{m} = \sqrt{k/62.832} \)

When spring constant is reduced, \( \omega_n \) decreases.

\[
\langle \omega \rangle_{\text{new}} = 0.55 \quad \omega_n = 34.5576 \, \text{rad/sec} = \sqrt{\frac{k_{\text{new}}}{m_{\text{new}}}} = \sqrt{\frac{k-800}{k}} \times 62.836 = 34.5576 \quad \sqrt{\frac{k-800}{k}} = 0.55
\]

\[
\frac{k-800}{k} = (0.55)^2 = 0.3025
\]

\[
k = 1146.9534 \, \text{N/m}
\]

\[
\sqrt{m} = \sqrt{k/62.832} \quad m = \frac{k}{62.832^2} = \frac{1146.9534}{3947.8602}
\]

\[
m = 0.2905 \, \text{kg}
\]

2.4 \( k = 100 \left(\frac{10}{1000}\right) = 10000 \, \text{N/m} \)

\[
\omega_n = \sqrt{\frac{k_g}{m}} = \sqrt{\frac{4k}{m}} = \left(\frac{4 \times 10^4}{10}\right)^{\frac{1}{2}} = 63.2456 \, \text{rad/sec}
\]

\[
\tau_n = \frac{2\pi}{\omega_n} = 6.2832 \frac{63.2456}{63.2456} = 0.0993 \, \text{sec}
\]
2.5 \[ m = \frac{2000}{386.4} \] 
Let \( \omega_n = 7.5 \text{ rad/sec} \).

\[ \omega_n = \sqrt{\frac{k_{eq}}{m}} \]

\[ k_{eq} = m \omega_n^2 = \left( \frac{2000}{386.4} \right) (7.5)^2 = 291.1491 \text{ lb/in} = 4 k \]

where \( k \) is the stiffness of the air spring.
Thus \( k = \frac{291.1491}{4} = 72.7873 \text{ lb/in} \).

2.6 
\[ x = A \cos (\omega_n t - \phi_0) \] 
\[ \dot{x} = -\omega_n A \sin (\omega_n t - \phi_0) \] 
\[ \ddot{x} = -\omega_n^2 A \cos (\omega_n t - \phi_0) \]

(a) \[ \omega_n A = 0.1 \text{ m/sec} ; \quad T_n = \frac{2\pi}{\omega_n} = 2 \text{ sec} \] 
\[ \omega_n = 3.1416 \text{ rad/sec} \]
\[ A = \frac{0.1}{\omega_n} = 0.03183 \text{ m} \]

(d) \[ x_0 = x(t = 0) = A \cos (-\phi_0) = 0.02 \text{ m} \]
\[ \cos(-\phi_0) = \frac{0.02}{A} = 0.6283 \]
\[ \phi_0 = 51.0724^\circ \]

(b) \[ \dot{x}_0 = \dot{x}(t = 0) = -\omega_n A \sin(-\phi_0) = -0.1 \sin(-51.0724^\circ) \]
\[ = 0.077779 \text{ m/sec} \]

(c) \[ \ddot{x} \big|_{\text{max}} = \omega_n^2 A = (3.1416)^2 (0.03183) = 0.314151 \text{ m/sec}^2 \]

2.7 
For small angular rotation of bar PQ about P,
\[ \frac{1}{2} (k_{12})_{eg} (\theta l_3)^2 = \frac{1}{2} k_1 (\theta l_1)^2 + \frac{1}{2} k_2 (\theta l_2)^2 \]
i.e., \[ (k_{12})_{eg} = \frac{k_1 l_1^2 + k_2 l_2^2}{l_3^2} \]

Let \( k_{eq} \) = overall spring constant at \( \theta \).

\[ \frac{1}{k_{eq}} = \frac{1}{(k_{12})_{eg}} + \frac{1}{k_3} \]

\[ k_{eq} = \frac{(k_{12})_{eg} k_3}{(k_{12})_{eg} + k_3} = \frac{k_3 (\frac{l_1}{l_3})^2 + k_2 (\frac{l_2}{l_3})^2}{k_1 (\frac{l_1}{l_3})^2 + k_2 (\frac{l_2}{l_3})^2 + k_3} \]
\[
\omega_n = \sqrt{\frac{k_{eq}}{m}} = \sqrt{\frac{k_1 k_2 l_1^2 + k_2 k_3 l_2^2}{m (k_1 l_1^2 + k_2 l_2^2 + k_3 l_3^2)}}
\]

2.8
\[
m = 2000 \text{ kg} \quad \delta_{st} = 0.02 \text{ m} \\
\omega_n = \left(\frac{g}{\delta_{st}}\right)^{1/2} = \left(\frac{9.81}{0.02}\right)^{1/2} = 22.1472 \text{ rad/sec}
\]

2.9
Let \( x \) be measured from the position of mass at which the springs are unstretched.

Equation of motion is
\[
m \dddot{x} = -k_1(x + \delta_{st}) - k_2(x + \delta_{st}) + W \sin \theta \quad (E_1)
\]
where \( \delta_{st} (k_1 + k_2) = W \sin \theta \).

Thus \( E_1 \) becomes \( m \dddot{x} + (k_1 + k_2) x = 0 \Rightarrow \omega_n = \sqrt{\frac{k_1 + k_2}{m}} \).

2.10
\[
k_1 = \frac{A_1 E_1}{l_1} = \frac{\pi}{4} (0.05)^2 (30 (10^6)) \\
= 163.6250 \text{ lb/in}
\]
\[
k_2 = \frac{A_2 E_2}{l_2} = \frac{163.6250}{25} = 196.35 \text{ lb/in}
\]
\[
k_{eq} = k_1 + k_2 = 163.6250 + 196.35 = 359.975 \text{ lb/in}
\]
Let \( x \) be measured from the unstretched length of the springs. The equation of motion is:
\[
m \dddot{x} = -k_1 (x + \delta_{st}) - k_2 (x + \delta_{st}) + W \sin \theta
\]
where \( (k_1 + k_2) \delta_{st} = W \sin \theta \)

i.e., \( m \dddot{x} + (k_1 + k_2) x = 0 \)

Thus the natural frequency of vibration of the cart is given by
\[
\omega_n = \sqrt{\frac{k_1 + k_2}{m}} = \sqrt{\frac{359.975 (386.4)}{5000}} = 5.2743 \text{ rad/sec}
\]

2.11
Weight of electronic chassis = 500 N. To be able to use the unit in a vibratory environment with a frequency range of 0 - 5 Hz, its natural frequency must be away from the frequency of the environment. Let the natural frequency be \( \omega_n = 10 \text{ Hz} = 62.832 \text{ rad/sec} \). Since
\[
\omega_n = \sqrt{\frac{k_{eq}}{m}} = 62.832
\]
we have

2-3
\[ k_{eq} = m \omega_n^2 = \left( \frac{500}{9.81} \right) (62.832)^2 = 20.1857 \times 10^4 \text{ N/m} = 4k \]

so that \( k \) = spring constant of each spring = 50,464.25 N/m. For a helical spring,

\[ k = \frac{G d^4}{8 \pi n D^3} \]

Assuming the material of springs as steel with \( G = 80 \times 10^9 \) Pa, \( n = 5 \) and \( d = 0.005 \) m, we find

\[ k = 50,464.25 = \frac{80 (10^9) (0.005)^4}{8 (5) D^3} \]

This gives

\[ D^3 = \frac{1250 (10^{-3})}{50464.25} = 24,770.0 (10^{-9}) \text{ or } D = 0.0291492 \text{ m} = 2.91492 \text{ cm} \]

2.12

(i) with springs \( k_1 \) and \( k_2 \):

Let \( y_a, y_b, y_l \) be deflections of beam at distances \( a, b, l \) from fixed end.

\[ \frac{1}{2} (k_{12})_{eq} y_l^2 = \frac{1}{2} k_1 y_a^2 + \frac{1}{2} k_2 y_b^2 \]

i.e.,

\[ (k_{12})_{eq} = k_1 \left( \frac{y_a}{y_l} \right)^2 + k_2 \left( \frac{y_b}{y_l} \right)^2 \]

\[ y = \frac{F x^2}{6EI} (3l - x) \]

@ \( x = a \), \( y_a = \frac{F a^2}{6EI} (3l - a) \)

@ \( x = b \), \( y_b = \frac{F b^2}{6EI} (3l - b) \)

@ \( x = l \), \( y_l = \frac{F l^3}{3EI} \)

\[ \omega_n = \left[ \frac{k_1 k_3 \left( \frac{y_a}{y_l} \right)^2 + k_2 k_3 \left( \frac{y_b}{y_l} \right)^2}{m \left\{ k_1 \left( \frac{y_a}{y_l} \right)^2 + k_2 \left( \frac{y_b}{y_l} \right)^2 + \kappa_{beam} \right\}} \right]^{\frac{1}{2}} \]

where \( \kappa_{beam} = \frac{3EI}{l^3} \)

\[ = \left[ \frac{k_1 (3EI) a^4 (3l - a)^2 + k_2 (3EI) b^4 (3l - b)^2}{m l^3 \left\{ k_1 a^4 (3l - a)^2 + k_2 b^4 (3l - b)^2 + 12EI l^3 \right\}} \right]^{\frac{1}{2}} \]

(ii) without springs \( k_1 \) and \( k_2 \):

\[ \omega_n = \sqrt{\frac{k_{beam}}{m}} = \sqrt{\frac{3EI}{m l^3}} \]

2-4
2.13 Let \( x_1, x_2 \) = displacements of pulleys 1, 2

\[
x = 2x_1 + 2x_2 \quad (E_1)
\]

Let \( P \) = tension in rope.

For equilibrium of pulley 1,

\[
2P = k_1x_1 \quad (E_2)
\]

For equilibrium of pulley 2,

\[
2P = k_2x_2 \quad (E_3)
\]

where \( \frac{1}{k_1} = \frac{1}{4k} + \frac{1}{4k} = \frac{1}{2k} \); \( k_1 = 2k \)

and \( k_2 = \frac{P}{x} = 2k \)

Combining Eqs. (E_1) to (E_3):

\[
x = 2x_1 + 2x_2 = 2 \left( \frac{2P}{k_1} \right) + 2 \left( \frac{2P}{k_2} \right) = 4P \left( \frac{1}{k} + \frac{1}{2k} \right) = \frac{4P}{k}
\]

Let \( k_{eq} = \frac{P}{m} \)

**Equation of motion of mass \( m \):**

\[
m \ddot{x} + k_{eq}x = 0
\]

\[
\omega_n = \sqrt{\frac{k_{eq}}{m}} = \sqrt{\frac{k}{4m}}
\]

2.14 For a displacement of \( x \) of mass \( m \), pulleys 1, 2 and 3 undergo displacements of 2\( x \), 4\( x \) and 8\( x \), respectively.

The equation of motion of mass \( m \) can be written as

\[
m \ddot{x} + F_0 = 0 \quad (1)
\]

where \( F_0 = 2F_1 = 4F_2 = 8F_3 \) as shown in figure.

Since \( F_3 = (8x)k \), Eq. (1) can be rewritten as

\[
m \ddot{x} + 8F_3 = 8(8k) = 0 \quad (2)
\]

from which we can find

\[
\omega_n = \sqrt{\frac{64k}{m}} = 8 \sqrt{\frac{k}{m}} \quad (3)
\]
(a) \( \omega_n = \sqrt{\frac{4k}{M}} \)

(b) \( \omega_n = \sqrt{\frac{4k}{(M + m)}} \)

**Initial conditions:**
- velocity of falling mass \( m = v = \sqrt{2gL} \) (\( \therefore \frac{v^2}{\omega_n^2} = 2gL \))
- \( x_0 = x(t=0) = \frac{\text{weight}}{k_{eq}} = -\frac{mg}{4k} \)

**Conservation of momentum:**
- \( (M + m) \ddot{x}_0 = m \ddot{v} = m \sqrt{2gL} \)
- \( x_0 = x(t=0) = \frac{m}{M + m} \sqrt{2gL} \)

**Complete solution:**
- \( x(t) = A_0 \sin(\omega_n t + \phi_0) \)
  where \( A_0 = \sqrt{\frac{x_0^2 + (\frac{\dot{x}_0}{\omega_n})^2}{\frac{m^2 g^2}{16k^2} + \frac{m^2 v^2}{2k(M + m)}}} \)
  and
  \( \phi_0 = \tan^{-1}\left(\frac{x_0 \omega_n}{\dot{x}_0}\right) = \tan^{-1}\left(\frac{-v}{\sqrt{4k(M + m)}}\right) \)

---

2.16 (a) Velocity of anvil \( v = 50 \) ft/sec \( = 600 \) in/sec. \( x = 0 \) at static equilibrium position.

- \( x_0 = x(t=0) = \frac{\text{weight}}{k_{eq}} = -\frac{mg}{4k} \)

**Conservation of momentum:**
- \( (M + m) \ddot{x}_0 = m \ddot{v} \) or \( \ddot{x}_0 = \dot{x}(t=0) = \frac{m v}{M + m} \)

**Natural frequency:**
- \( \omega_n = \sqrt{\frac{4k}{M + m}} \)

**Complete solution:**
- \( x(t) = A_0 \sin(\omega_n t + \phi_0) \)
  where
  \[ A_0 = \sqrt{\frac{\dot{x}_0^2 + \left(\frac{\ddot{x}_0}{\omega_n}\right)^2}{\frac{m^2 g^2}{16k^2} + \frac{m^2 v^2}{(M + m)}}} \]
  and
  \[ \phi_0 = \tan^{-1}\left(\frac{x_0 \omega_n}{\dot{x}_0}\right) = \tan^{-1}\left(-\frac{mg}{4k} \sqrt{\frac{4k}{(M + m)}}\right) \]
Since $v = 600$, $m = \frac{12}{386.4}$, $M = \frac{100}{386.4}$, $k = 100$, we find

$$A_0 = \left\{ \left( \frac{12}{4} \right) \frac{(386.4)}{100} \right\}^2 + \left( \frac{12}{386.4} \right)^{\frac{1}{2}} = 1.7308 \text{ in}$$

$$\phi_0 = \tan^{-1} \left( -\frac{386.4 \sqrt{112}}{\sqrt{386.4} \ (600) \sqrt{400}} \right) = \tan^{-1} (-0.01734) = -0.9934 \text{ deg}$$

(b) $x = 0$ at static equilibrium position: $x_0 = x(t=0) = 0$. Conservation of momentum gives:

$$M \dot{x}_0 = m v \quad \text{or} \quad \dot{x}_0 = \ddot{x}(t=0) = \frac{m v}{M}$$

Complete solution:

$$x(t) = A_0 \sin (\omega_n t + \phi_0)$$

where

$$A_0 = \left\{ \frac{x_0^2}{\omega_n^2} \right\}^{\frac{1}{2}} = \left( \frac{m^2 v^2 (M)}{M^2 4 k} \right)^{\frac{1}{2}} = \frac{m v}{\sqrt{4 k M}} = \frac{12}{386.4} \frac{v (100) (100)}{\sqrt{400}} = 1.8314 \text{ in}$$

$$\phi_0 = \tan^{-1} \left( \frac{x_0 \omega_n}{k_0} \right) = \tan^{-1} (0) = 0$$

### 2.17

$$k_2 = \frac{3 E I_1}{L_i^3} \quad \text{(at tip)} ; \quad k_2 = \frac{12 E_2 I_2}{L_2^3} \quad \text{(at middle)}$$

$$k_{\text{eff}} = k_1 + k_2$$

$$\omega_n = \sqrt{\frac{k_{\text{eff}}}{m}} = \sqrt{\frac{3 E I_1}{L_i^3} + \frac{12 E_2 I_2}{L_2^3}} \frac{J}{W}$$

### 2.18

$$k = \frac{AE}{L} = \left\{ \frac{\pi}{4} (0.01)^2 \right\} \left\{ 2.07 \times 10^4 \right\} = 0.8129 \times 10^6 \text{ N/m}$$

$$m = 1000 \text{ kg}$$

$$\omega_n = \sqrt{\frac{k}{m}} = \left( \frac{0.8129 \times 10^6}{1000} \right)^{\frac{1}{2}} = 28.5114 \text{ rad/sec}$$

$$\dot{x}_0 = 2 \text{ m/s}, \quad x_0 = 0 \quad \text{(suddenly stopped while it has velocity)}$$

Period of ensuing vibration $= T_n = \frac{2\pi}{\omega_n} = \frac{2\pi}{28.5114} = 0.2204 \text{ sec}$

Amplitude $= A = \frac{\dot{x}_0}{\omega_n} = 2/28.5114 = 0.07015 \text{ m}$

### 2.19

$$\omega_n = 2 \text{ Hz} = 12.5664 \text{ rad/sec} = \sqrt{k/m}$$

$$\sqrt{k} = 12.5664 \text{ m/sec}$$

$$\omega_n' = \sqrt{\frac{k'}{m'}} = \sqrt{\frac{k}{m + 1}} = 6.2832 \text{ rad/sec}$$
\[
\sqrt{\kappa} = 6.2832 \sqrt{m+1} \\
= 12.5664 \sqrt{m} \\
\sqrt{m+1} = 2 \sqrt{m} \quad , \quad m = \frac{4}{3} \ \text{kg}
\]

\[
\kappa = (12.5664)^2 m = 52.6381 \ \text{N/m}
\]

2.20 Cable stiffness \( k = \frac{AE}{\ell} = \frac{1}{4} \left( \frac{\pi}{4} (0.01)^2 \right) 2.07 \times 10^{11} = 4.0644 \times 10^6 \ \text{N/m} \)

\[
\tau_n = 0.1 = \frac{1}{f_n} = \frac{2 \pi}{\omega_n}
\]

\[
\omega_n = \frac{2 \pi}{0.1} = 20 \pi = \sqrt{\frac{k}{m}}
\]

Hence

\[
m = \frac{k}{\omega_n^2} = \frac{4.0644 \times 10^6}{(20 \pi)^2} = 1029.53 \ \text{kg}
\]

2.21 \( b = 2 \ell \sin \theta \)

Neglect masses of links.

(a) \( k_{eq} = \kappa \left( \frac{4 \ell^2 - b^2}{b^2} \right) = \kappa \left( \frac{4 \ell^2 - 4 \ell^2 \sin^2 \theta}{4 \ell^2 \sin^2 \theta} \right) \)

\[
\omega_n = \sqrt{\frac{k_{eq}}{m}} = \sqrt{\frac{k \ell^2 \cos^2 \theta}{m}} \quad \text{from solution of problem 1.8}
\]

(b) \( \omega_n = \sqrt{\frac{k \ell^2}{m}} \quad \text{since} \quad k_{eq} = \kappa \).

2.22 \( y = \sqrt{l^2 - (l \sin \theta - x)^2} - l \cos \theta = \sqrt{l^2 (\cos^2 \theta + \sin^2 \theta) - (l \sin \theta - x)^2} - l \cos \theta \)

\[
= \ell \cos \theta \sqrt{1 - \frac{x^2}{\ell^2 \cos^2 \theta} + \frac{2 \ell \sin \theta}{\ell^2 \cos^2 \theta} - \ell \cos \theta}
\]

\[
\frac{1}{2} k_{eq} x^2 = \frac{1}{2} k_1 y^2 + \frac{1}{2} k_2 y^2
\]

with \( y \approx \ell \cos \theta \left( 1 - \frac{1}{2} \frac{x^2}{\ell^2 \cos^2 \theta} + \frac{1}{2} \frac{2 \ell \sin \theta}{\ell^2 \cos^2 \theta} \right) - \ell \cos \theta \)

\[
\approx \frac{x \sin \theta}{\cos \theta} = x \tan \theta \quad \text{(since} \ x^2 << x, \ \text{it is neglected)}
\]
Thus $k_{eq}$ can be expressed as

$$k_{eq} = (k_1 + k_2) \tan^2 \theta$$

Equation of motion:

$$m \ddot{x} + k_{eq} x = 0$$

Natural frequency:

$$\omega_n = \sqrt{\frac{k_{eq}}{m}} = \sqrt{\frac{(k_1 + k_2) g}{W} \tan \theta}$$

\[\Box\text{(a)}\] Neglect masses of rigid links. Let $x = \text{displacement of W}$. Springs are in series.

$$k_{eq} = \frac{k}{2}$$

Equation of motion:

$$m \ddot{x} + k_{eq} x = 0$$

Natural frequency:

$$\omega_n = \sqrt{\frac{k_{eq}}{m}} = \sqrt{\frac{k}{2 m}}$$

\[\Box\text{(b)}\] Under a displacement of $x$ of mass, each spring will be compressed by an amount:

$$x_s = x \frac{2}{b} \sqrt{\ell^2 - \frac{b^2}{4}}$$

Equivalent spring constant:

$$\frac{1}{2} k_{eq} x^2 = 2 \left( \frac{1}{2} k x_s^2 \right)$$

or

$$k_{eq} = 2 k \left( \frac{x_s}{x} \right)^2 = 2 k \left( \frac{4}{b^2} \right) \left( \ell^2 - \frac{b^2}{4} \right) = \frac{8 k}{b^2} \left( \ell^2 - \frac{b^2}{4} \right)$$

Equation of motion:

$$m \ddot{x} + k_{eq} x = 0$$

Natural frequency:

$$\omega_n = \sqrt{\frac{k_{eq}}{m}} = \sqrt{\frac{8 k}{b^2 m} \left( \ell^2 - \frac{b^2}{4} \right)}$$
2.24 \[ F_1 = F_3 = k_1 \cos 45^\circ \]
\[ F_2 = F_4 = k_2 \cos 135^\circ \]
\[ F = F_1 \cos 45^\circ + F_2 \cos 135^\circ + F_3 \cos 45^\circ + F_4 \cos 135^\circ = 2 \alpha (k_1 \cos^2 45^\circ + k_2 \cos^2 135^\circ) \]
\[ k_{eq} = \frac{F}{\alpha} = 2 \left( \frac{k_1}{2} + \frac{k_2}{2} \right) = k_1 + k_2 \]
Equation of motion:
\[ m \ddot{x} + (k_1 + k_2) \alpha = 0 \]

2.25 Let \( \alpha_i \) denote the angle made by \( i \)th spring with respect to \( x \) axis.
Let \( x \) = displacement of mass along the direction defined by \( \Theta \).

If \( k_{eq} \) = equivalent spring constant, the equivalence of potential energies gives
\[ \frac{1}{2} k_{eq} \alpha^2 = \frac{1}{2} \sum_{i=1}^{6} k_i \left\{ x \cos (\Theta - \alpha_i) \right\}^2 \]
\[ k_{eq} = \sum_{i=1}^{6} k_i \cos^2 (\Theta - \alpha_i) = \sum_{i=1}^{6} k_i \left( \cos \Theta \cos \alpha_i + \sin \Theta \sin \alpha_i \right)^2 \]
\[ = \sum_{i=1}^{6} k_i \left( \cos^2 \alpha_i \cos^2 \Theta + \sin^2 \alpha_i \sin^2 \Theta \right) \]
\[ + 2 \sum_{i=1}^{6} \left( \cos \alpha_i \sin \alpha_i \cos \Theta \sin \Theta \right) \]

Natural frequency = \( \omega_n = \sqrt{\frac{k_{eq}}{m}} \)

For \( \omega_n \) to be independent of \( \Theta \),
\[ \sum_{i=1}^{6} k_i \cos \alpha_i = \sum_{i=1}^{6} k_i \sin \alpha_i \quad \text{(Eq. 1)} \]
\[ \text{and} \quad \sum_{i=1}^{6} k_i \cos \alpha_i \sin \alpha_i = 0 \quad \text{(Eq. 2)} \]

(Eq. 1) and (Eq. 2) can be rewritten as
\[ \sum_{i=1}^{6} \frac{1}{2} k_i \left( \frac{1}{2} + \frac{1}{2} \cos 2\alpha_i \right) = \sum_{i=1}^{6} k_i \left( \frac{1}{2} - \frac{1}{2} \cos 2\alpha_i \right) \]
\[ \text{and} \quad \frac{1}{2} \sum_{i=1}^{6} k_i \sin 2\alpha_i = 0 \]
\[ \text{i.e.} \quad \sum_{i=1}^{6} k_i \cos 2\alpha_i = 0 \quad \text{--- (Eq. 1)} \]
\[ \text{and} \quad \sum_{i=1}^{6} k_i \sin 2\alpha_i = 0 \quad \text{--- (Eq. 2)} \]
In the present example, \((E_3)\) and \((E_4)\) become

\[
k_1 \cos 60^\circ + k_2 \cos 240^\circ + k_3 \cos 2\alpha_3 + k_1 \cos 420^\circ + k_2 \cos 60^\circ = 0
\]

\[
k_1 \sin 60^\circ + k_2 \sin 240^\circ + k_3 \sin 2\alpha_3 + k_1 \sin 420^\circ + k_2 \sin 60^\circ = 0
\]

\[
\frac{k_1 - k_2 + 2 k_3 \cos 2\alpha_3 = 0}{k_1 - k_2 + 2 k_3 \sin 2\alpha_3 = 0}
\]

\[
\frac{\sqrt{3} k_1 - \sqrt{3} k_2 + 2 k_3 \cos 2\alpha_3 = 0}{\sqrt{3} k_1 - \sqrt{3} k_2 + 2 k_3 \sin 2\alpha_3 = 0}
\]

Squaring \((E_5)\) and \((E_6)\) and adding,

\[
4 k_3^2 = (k_2 - k_1)^2 (1 + 3)
\]

\[
\implies k_3 = \pm \frac{k_2 - k_1}{\sqrt{3}} \Rightarrow k_3 = \left| \frac{k_2 - k_1}{\sqrt{3}} \right|
\]

Dividing \((E_6)\) by \((E_5)\),

\[
\tan 2\alpha_3 = \frac{\sqrt{3}}{3}
\]

\[
\implies \alpha_3 = \frac{1}{2} \tan^{-1} \left( \frac{\sqrt{3}}{3} \right) = 30^\circ
\]

\[\Box\]

\[T_1 = \frac{F}{a}, \quad T_2 = \frac{F}{b} \] (a)

\[m \ddot{x} + (T_1 + T_2) = 0\]

\[m \ddot{x} + \left( \frac{T}{a} + \frac{T}{b} \right) x = 0\]

(b) \(\omega_n = \sqrt{\frac{T}{m a b}} = \sqrt{\frac{T}{m a b}} \left( a+b \right)\)

\[\Box\]

\[m = \frac{160}{386.4} \text{ lb-sec}^2/\text{inch}, \quad k = 10 \text{ lb/inch}.
\]

Velocity of jumper as he falls through 200 ft:

\[m g h = \frac{1}{2} m v^2 \quad \text{or} \quad v = \sqrt{2 g h} = \sqrt{2 \times 386.4 \times 200 (12)} = 1,361.8811 \text{ in/sec}\]

About static equilibrium position:

\[x_0 = x(t=0) = 0, \quad \dot{x}_0 = \ddot{x}(t=0) = 1,361.8811 \text{ in/sec}\]

Response of jumper:

\[x(t) = A_0 \sin (\omega_n t + \phi_0)\]

where

\[A_0 = \left( x_0^2 + \left( \frac{\dot{x}_0}{\omega_n} \right)^2 \right)^{\frac{1}{2}} = \frac{\dot{x}_0}{\omega_n} = \frac{\dot{x}_0 \sqrt{m}}{\sqrt{k}} = \frac{1361.8811}{\sqrt{10}} \sqrt{\frac{160}{386.4}} = 277.1281 \text{ in}\]

and

\[\phi_0 = \tan^{-1} \left( \frac{x_0 \omega_n}{\dot{x}_0} \right) = 0\]
The natural frequency of a vibrating rope is given by (see Problem 2.28):

$$\omega_n = \sqrt{\frac{T(a + b)}{ma}}$$

where $T =$ tension in rope, $m =$ mass, and $a$ and $b$ are lengths of the rope on both sides of the mass. For the given data:

$$10 = \left( \frac{T(80 + 160)}{120} \right)^{\frac{1}{2}} = \sqrt{T(0.060375)}$$

which yields

$$T = \frac{100}{0.060375} = 1,656.3147 \text{ lb}$$

2.29 when $\omega = 0$, total vertical height $= 2l + h$

when $\omega \neq 0$, total vertical height $= (2l \cos \theta + h)$

spring force $= k[2l + h - (2l \cos \theta + h)]$  

$= 2k l (1 - \cos \theta)$

For vertical equilibrium of mass $m$,

$$mg + T_2 \cos \theta = T_1 \cos \theta \quad \text{--- (E1)}$$

For horizontal equilibrium,

$$T_2 = \left( \frac{F_c - T_1 \sin \theta}{\sin \theta} \right) \sin \theta \quad \text{--- (E2)}$$

From (E2), (E1) can be expressed as

$$mg + \left( \frac{F_c - T_1 \sin \theta}{\sin \theta} \right) \cos \theta = T_1 \cos \theta$$

i.e.,

$$T_1 = \frac{mg + F_c \cot \theta}{\cos \theta} = \frac{mg + m \omega^2 l \cos \theta}{2 \cos \theta}$$

$$T_2 = \frac{F_c - T_1 \sin \theta}{\sin \theta} = \frac{m \omega^2 l \cos \theta - mg}{2 \cos \theta}$$

spring force $= 2k l (1 - \cos \theta) = 2 T_2 \cos \theta$  

$$= m l \omega^2 \cos \theta - mg$$

$$\cos \theta = \left( \frac{2k l + mg}{2k l + m l \omega^2} \right) \quad \text{--- (E3)}$$
This equation defines the equilibrium position of mass \( m \).

For small oscillations about the equilibrium position, Newton's second law gives

\[
2m \ddot{y} + k y = 0, \quad \omega_n = \sqrt{\frac{2k}{m}}
\]

2.30 (a) Let \( P \) = total spring force, \( F \) = centrifugal force acting on each ball. Equilibrium of moments about the pivot of bell crank lever (O) gives:

\[
F \left( \frac{20}{100} \right) = \frac{P}{2} \left( \frac{12}{100} \right)
\]

When \( P = 10^4 \left( \frac{1}{100} \right) = 100 \) N, and

\[
F = m r \omega^2 = m r \left( \frac{2 \pi N}{60} \right)^2 = \frac{25}{9.81} \left( \frac{16}{100} \right) \left( \frac{2 \pi N}{60} \right)^2 = 0.004471 \text{ N}^2
\]

where \( N \) = speed of the governor in rpm. Equation (1) gives:

\[
0.004471 \text{ N}^2 \cdot 0.2 = \frac{100}{2} \cdot 0.12 \quad \text{or} \quad N = 81.9140 \text{ rpm}
\]

(b) Consider a small displacement of the ball arm about the vertical position. Equilibrium about point O gives:

\[
(m b^2) \ddot{\theta} + (k a \sin \theta) a \cos \theta = 0
\]

For small values of \( \theta \), \( \sin \theta \approx \theta \) and \( \cos \theta \approx 1 \), and hence Eq. (2) gives

\[
m b^2 \ddot{\theta} + k a^2 \theta = 0
\]

from which the natural frequency can be determined as

\[
\omega_n = \left( \frac{k a^2}{m b^2} \right)^{\frac{1}{2}} = \left( 10^4 \left( \frac{0.12}{0.20} \right) \frac{9.81}{25} \right)^{\frac{1}{2}} = 37.5851 \text{ rad/sec}
\]
2.31

\[ \text{so} = \frac{\alpha}{\sqrt{2}}, \quad \text{so} = h, \quad \text{os} = \sqrt{h^2 + \frac{a^2}{2}} \]

When each wire stretches by \( x \), let the resulting vertical displacement of the platform be \( \Delta \).

\[ \Delta = \sqrt{h^2 + \frac{a^2}{2}} \left\{ \sqrt{ \left( \frac{h+x}{\sqrt{h^2 + \frac{a^2}{2}}} \right)^2 + \frac{a^2}{2}} - 1 \right\} \]

\[ = \sqrt{h^2 + \frac{a^2}{2}} \left[ \sqrt{1 + \left( \frac{2hx + x^2}{h^2 + \frac{a^2}{2}} \right)} - 1 \right] \]

For small \( x \), \( x^2 \) is negligible compared to \( 2hx \) and \( \sqrt{1 + \theta} \approx 1 + \frac{\theta}{2} \) and hence

\[ x = \frac{\sqrt{h^2 + \frac{a^2}{2}} \left[ 1 + \frac{h}{x^2} \right]}{\left( h^2 + \frac{a^2}{2} \right)} \]

Potential energy equivalence gives

\[ \frac{1}{2} k_{eq} x^2 = 4 \left( \frac{1}{2} k x^2 \right) \]

\[ k_{eq} = 4 k \left( \frac{x_1}{x} \right)^2 = \frac{4 k h^2}{(h^2 + \frac{a^2}{2})} \]

Equation of motion of M:

\[ \sum F = ma \]

\[ M \ddot{x} + k_{eq} x = 0 \]

\[ \tau_n = \frac{2\pi}{\omega_n} = \frac{2\pi}{\left( \frac{k_{eq}}{M} \right)^{1/2}} = \frac{\pi \sqrt{M}}{h} \left( \frac{2 h^2 + a^2}{2 k} \right)^{1/2} \]

2.32

Equation of motion:

\[ m \ddot{x} = \sum F_x \]

i.e., \( (L \cdot A \cdot \rho) \ddot{x} = -2 (A \times \rho \cdot g) \)

i.e., \( \ddot{x} + \frac{2g}{L} x = 0 \)

Where \( A = \) cross-sectional area of the tube and \( \rho = \) density of mercury. Thus the natural frequency is given by:

\[ \omega_n = \sqrt{\frac{2g}{L}} \]
Assume same area of cross section for all segments of the cable. Speed of blades = 300 rpm = 5 Hz = 31.416 rad/sec.

\[ \omega_n^2 = \frac{k_{eq}}{m} = (2(31.416))^2 = (62.832)^2 \]

\[ k_{eq} = m \omega_n^2 = 250 \times 62.832^2 = 98.6965 \times 10^4 \text{ N/m} \]

\[ AD = \sqrt{0.5^2 + 0.5^2} = 0.7071 \text{ m}, \quad OD = \sqrt{2^2 + 0.7071^2} = 2.1213 \text{ m} \]

Stiffness of cable segments:

\[ k_{PO} = \frac{AE}{\ell_{PO}} = \frac{A(207)(10^9)}{1} = 207 \times 10^9 \text{ N/m} \]

\[ K_{OD} = \frac{AE}{\ell_{OD}} = \frac{A(207)(10^9)}{2.1213} = 97.5817 \times 10^9 \text{ N/m} \]

The total stiffness of the four inclined cables \((k_{ic})\) is given by:

\[ k_{ic} = 4 k_{OD} \cos^2 \theta = 4 \times (97.5817 \times 10^9) \cos^2 19.471^\circ = 346.9581 \times 10^9 \text{ N/m} \]

Equivalent stiffness of vertical and inclined cables is given by:

\[ \frac{1}{k_{eq}} = \frac{1}{k_{PO}} + \frac{1}{k_{ic}} = \frac{k_{PO}}{k_{PO} + k_{ic}} \]

\[ k_{eq} = \frac{k_{PO}}{k_{PO} + k_{ic}} = \frac{(207 \times 10^9)A \times (346.9581 \times 10^9)A}{(207 \times 10^9)A + (346.9581 \times 10^9)A} = 129.6494 \times 10^9 \text{ A N/m} \]

Equating \(k_{eq}\) given by Eqs. (1) and (2), we obtain the area of cross section of cables as:

\[ A = \frac{98.6965 \times 10^4}{129.6494 \times 10^9} = 7.6126 \times 10^{-6} \text{ m}^2 \]

2-15
\[ \frac{1}{2 \pi} \left( \frac{k_1}{m} \right)^{\frac{1}{2}} = 5 ; \quad \frac{k_1}{m} = 4 (\pi)^2 (25) = 986.9651 \]

\[ \frac{1}{2 \pi} \left( \frac{k_1}{m + 5000} \right)^{\frac{1}{2}} = 4.0825 ; \quad \frac{k_1}{m + 5000} = 4 (\pi)^2 (16.6668) = 657.9822 \]

Using \( k_1 = \frac{AE}{\varepsilon_1} \), we obtain

\[ \frac{k_1}{m} = \frac{AE}{\varepsilon_1 m} = \frac{A(207)(10^9)}{2 m} = 986.9651 \]

i.e., \( A = 9.5359 \times 10^{-9} \) m \( (1) \)

Also

\[ \frac{k_1}{m + 5000} = \frac{AE}{\varepsilon_1 (m + 5000)} = 657.9822 \]

i.e., \( \frac{A}{m + 5000} = 6.3573 \times 10^{-9} \) \( (2) \)

Using Eqs. (1) and (2), we obtain

\[ A = 9.5359 \times 10^{-9} \) m + 31.7865 \times 10^{-9} \] m

i.e., \( 31.786 \times 10^{-9} \) m = 31.7865 \times \( \) \( (3) \)

\[ m = 10000.1573 \text{ kg} \]

Equations (1) and (3) yield

\[ A = 9.5359 \times 10^{-9} \) m = 9.5359 \times 10^{-9} \) \( \times 10000.1573 = 0.9536 \times 10^{-4} \) \( m^2 \]

### Longitudinal Vibration:

Let \( W_1 = \text{part of weight } w \text{ carried by length } a \text{ of shaft} \)

\( W_2 = W - W_1 = \text{weight carried by length } b \)

\[ x = \text{Elongation of length } a = \frac{W_1 a}{AE} \]

\[ y = \text{shortening of length } b = \frac{(W - W_1) (l - a)}{AE} \]

Since \( x = y \),

\[ W_1 = \frac{W (l - a)}{l} \]

\[ x = \text{elongation or static deflection of length } a = \frac{W a (l - a)}{AE l} \]

Considering the shaft of length \( a \) with end mass \( W_1 / g \) as a spring-mass system,

\[ \omega_n = \sqrt{\frac{g}{x}} = \left( \frac{g l AE}{W a (l - a)} \right)^{1/2} \]

2-16
Transverse vibration:

spring constant of a fixed-fixed beam with off-center load

\[ k = \frac{3EI}{a^3b^3} = \frac{3EI}{a^3(l-a)^3} \]

\[ \omega_n = \sqrt{\frac{k}{m}} = \left\{ \frac{3EI}{W a^3(l-a)^3} \right\}^{1/2} \]

with \( I = \left( \frac{\pi d^4}{64} \right) \)  

moment of inertia

Torsional vibration:

If flywheel is given an angular deflection \( \theta \), resisting torques offered by lengths \( a \) and \( b \) are \( \frac{GJ\theta}{a} \) and \( \frac{GJ\theta}{b} \).

Total resisting torque = \( M_t = GJ\left( \frac{1}{a} + \frac{1}{b} \right) \theta \)

\[ k_t = \frac{M_t}{\theta} = GJ\left( \frac{1}{a} + \frac{1}{b} \right) \]

where \( J = \frac{\pi d^4}{32} \) = polar moment of inertia

\[ \omega_n = \sqrt{\frac{k_t}{J_o}} = \left\{ \frac{GJ}{J_o} \left( \frac{1}{a} + \frac{1}{b} \right) \right\}^{1/2} \]

where \( J_o = \) mass polar moment of inertia of the flywheel.

\[ 2.36 \quad m_{eq_end} = \text{equivalent mass of a uniform beam at the free end (see Problem 2.38)} = \]

\[ \frac{33}{140} \quad m = \frac{33}{140} \left( 1 \left( 150 \times 12 \right) \frac{0.283}{386.4} \right) = 0.3107 \]

Stiffness of tower (beam) at free end:

\[ k_b = \frac{3EI}{L^3} = \frac{3 \left( 30 \times 10^6 \right) \left( \frac{1}{12} \right) \left( 1 \right) \left( 1^3 \right)}{(150 \times 12)^3} = 0.001286 \text{ lb/in} \]

Length of each cable:

\[ OA = \sqrt{2} = 1.4142 \text{ ft} , \quad OB = \sqrt{2} = 15 = 21.2132 \text{ ft} , \quad AB = OB - OA = 19.7990 \text{ ft} \]

\[ TB = \sqrt{TA^2 + AB^2} = \sqrt{100^2 + 19.7990^2} = 101.9412 \text{ ft} \]

\[ \tan \theta = \frac{AT}{AB} = \frac{100}{19.7990} = 5.0508 , \quad \theta = 78.8008^\circ \]

Axial stiffness of each cable:

\[ k = \frac{AE}{\ell} = \frac{(0.5) \left( 30 \times 10^6 \right)}{(101.9412 \times 12)} = 12261.971 \text{ lb/in} \]

Axial extension of each cable \( (y_c) \) due to a horizontal displacement of \( x \) of tower:
\[ \ell_1^2 = \ell^2 + x^2 - 2 \ell x \cos(180^\circ - \theta) = \ell^2 + x^2 \left(1 + \left(\frac{x}{\ell}\right)^2 + 2 \frac{x}{\ell} \cos \theta\right) \]

or
\[ \ell_1 = \ell \left(1 + \left(\frac{x}{\ell}\right)^2 + 2 \frac{x}{\ell} \cos \theta\right) \]

\[ \tau_c = \ell_1 - \ell = \ell \left(1 + \frac{1}{2} \frac{x^2}{\ell^2} + \frac{1}{2} (2) \frac{x}{\ell} \cos \theta\right) - \ell = \ell + x \cos \theta - \ell = x \cos \theta \]

Equivalent stiffness of each cable, \( k_{e_q,OB} \), in a horizontal direction, parallel to OAB, is given by
\[ \frac{1}{2} k_{y_c}^2 = \frac{1}{2} k_{e_q,OB} x^2 \quad \text{or} \quad k_{e_q,OB} = k \left(\frac{y_c}{x}\right)^2 = k \cos^2 \theta \]

Equivalent stiffness of each cable, \( k_{e_q,x} \), in a horizontal direction, parallel to the x-axis (along OS), can be found as
\[ k_{e_q,x} = k_{e_q,OB} \cos^2 45^\circ = \frac{1}{2} k_{e_q,OB} = \frac{1}{2} k \cos^2 \theta \]

(since angle BOS is 45°)

This gives
\[ k_{e_q,x} = \frac{1}{2} (12261.971 \cos^2 78.8008^\circ) = 231.2709 \text{ lb/in} \]

In order to use the relation \( k_{e_q,ad} = k_b + 4 k_{e_q,x} \left(\frac{y_{L1}}{y_L}\right)^2 \), we find
\[ \frac{y_{L1}}{y_L} = \frac{F L_1^2 (3 L - L_{11})}{6 E I} = \frac{3 E I L_1^2 (3 L - L_{11})}{100^2 (3 (150) - 100)} = 0.5185 \]

Thus
\[ k_{e_q,ad} = k_b + 4 k_{e_q,x} (0.5185)^2 = 0.001286 + 4 (231.2709)(0.5185)^2 = 248.7015 \text{ lb/in} \]

Natural frequency:
\[ \omega_n = \sqrt{\frac{k_{e_q,ad}}{m_{eq}} \left(\frac{1}{2}\right)^2} = 28.2923 \text{ rad/sec} \]
2.37  Sides of the sign:

\[
AB = \sqrt{8.8^2 + 8.8^2} = 12.44 \text{ in} \quad \text{; } BC = 30 - 8.8 - 8.8 = 12.4 \text{ in}
\]

\[
\text{Area} = 30 (30) - 4 \left( \frac{1}{2} (8.8) (8.8) \right) = 745.12 \text{ in}^2
\]

\[
\text{Thickness} = \frac{1}{8} \text{ in} \quad \text{; } \text{Weight density of steel} = 0.283 \text{ lb/in}^3
\]

Weight of sign = \((0.283)\left(\frac{1}{8}\right)(745.12) = 26.64 \text{ lb}\)

Weight of sign post = \((72) (2) \left( \frac{1}{4} \right) (0.283) = 10.19 \text{ lb}\)

Stiffness of sign post (cantilever beam):

\[
k = \frac{3 \ E \ I}{\epsilon^3}
\]

Area moments of inertia of the cross section of the sign post:

\[
I_{xx} = \frac{1}{12} (2) \left( \frac{1}{4} \right)^3 = \frac{1}{384} \text{ in}^4
\]

\[
I_{yy} = \frac{1}{12} \left( \frac{1}{4} \right) (2)^3 = \frac{1}{6} \text{ in}^4
\]

Bending stiffnesses of the sign post:

\[
k_{xx} = \frac{3 \ E \ I_{yy}}{\epsilon^3} = \frac{3 (30 (10^6)) (\frac{1}{6})}{72^3} = 40.1877 \text{ lb/in}
\]

\[
k_{yy} = \frac{3 \ E \ I_{xx}}{\epsilon^3} = \frac{3 (30 (10^6)) (\frac{1}{384})}{72^3} = 0.6279 \text{ lb/in}
\]

Torsional stiffness of the sign post:

2-19
\[ k_t = 5.33 \frac{a b^3}{\ell} G \left\{ 1 - 0.63 \frac{b}{a} \left( 1 - \frac{b^4}{12 a^4} \right) \right\} \]

Thus
\[ k_t = 5.33 \left\{ \frac{1}{72} \left( \frac{1}{8} \right)^3 \right\} \left( 11.5 \times 10^5 \right) \left\{ 1 - (0.63) \left( \frac{1}{8} \right) \left( 1 - \frac{1}{12 (1)^4} \right) \right\} \]
\[ = 1531.7938 \text{ lb-in/rad} \]

Natural frequency for bending in \( xx \) plane:
\[ \omega_{xx} = \left( \frac{k_{xx}}{m} \right)^{\frac{1}{2}} = \left( \frac{40.1877}{26.64} \right)^{\frac{1}{2}} = 24.1434 \text{ rad/sec} \]

Natural frequency for bending in \( yy \) plane:
\[ \omega_{yy} = \left( \frac{k_{yy}}{m} \right)^{\frac{1}{2}} = \left( \frac{0.6279}{26.64} \right)^{\frac{1}{2}} = 3.0178 \text{ rad/sec} \]

By approximating the shape of the sign as a square of side 30 in (instead of an octagon), we can find its mass moment of inertia as:
\[ I_{oo} = \gamma \frac{L^3}{3} (b^3 + h^3) = 0.283 \left( \frac{30}{386.4} \right) \left( 30^3 \left( \frac{1}{8} \right) + \left( \frac{1}{8} \right)^3 30 \right) = 24.7189 \]

Natural torsional frequency:
\[ \omega_t = \left( \frac{k_t}{I_{oo}} \right)^{\frac{1}{2}} = \left( \frac{1531.7938}{24.7189} \right)^{\frac{1}{2}} = 7.8720 \text{ rad/sec} \]

Thus the mode of vibration (close to resonance) is torsion in \( xy \) plane.

\[ \text{(a) Pivoted:} \]
\[ K_{eq} = 4 \quad K_{coulumn} = 4 \left( \frac{3 E I}{L^3} \right) = \frac{12 E I}{L^3} \]
Let \( m_{eff} = \text{effective mass due to self weight of columns} \)
Equation of motion: \( \left( \frac{w}{g} + m_{eff} \right) \ddot{x} + K_{eq} x = 0 \)
\[ \text{Natural frequency of horizontal vibration} = \frac{12 E I}{V^3 \left( \frac{w}{g} + m_{eff} \right)} \]
(b) Fixed:

Since the joint between column and floor does not permit rotation, each column will bend with inflection point at middle.

When force $F$ is applied at ends,

$$x = 2 \frac{F \left( \frac{h}{2} \right)^3}{3EI} = \frac{Fl^3}{12EI}$$

$$k_{\text{column}} = \frac{12EI}{l^3} \quad k_{\text{e}} = 4k_{\text{column}} = \frac{48EI}{l^3}$$

Let $m_{\text{eff2}}$ = effective mass of each column at top end

Equation of motion: $$\left( \frac{W}{g} + m_{\text{eff2}} \right) \ddot{x} + \frac{48EI}{l^3} x = 0$$

Natural frequency of horizontal vibration $= \omega_n = \sqrt{\frac{48EI}{l^3} \left( \frac{W}{g} + m_{\text{eff2}} \right)}$

Effective mass (due to self weight):

(a) Let $m_{\text{eff1}}$ = effective part of mass of beam ($m$) at end.

Thus vibrating inertia force at end is due to $(M + m_{\text{eff1}})$.

Assume deflection shape during vibration same as the static deflection shape with a tip load:

$$y(x,t) = Y(x) \cos(\omega_n t - \phi) \quad \text{where} \quad Y(x) = \frac{F \left( \frac{3l}{2} - x \right)^2}{6EI}$$

$$Y(x) = \frac{Y_0}{2l^3} \left( 3 \frac{x^2}{l} - x^3 \right) \quad \text{where} \quad Y_0 = \frac{Fl^3}{3EI} = \text{max. tip deflection}$$

$$y(x,t) = \frac{Y_0}{2l^3} \left( 3 \frac{x^2}{l} - x^3 \right) \cos(\omega_n t - \phi) \tag{E_1}$$

Max. strain energy of beam = Max. work by force $F$

$$= \frac{1}{2} F Y_0 = \frac{3}{2} \frac{EI}{l^3} Y_0^2 \tag{E_2}$$

Max. kinetic energy due to distributed mass of beam

$$= \frac{1}{2} \frac{m}{l} \int_0^l \dot{y}^2(x,t) \left. \right|_{\text{max}} dx + \frac{1}{2} \left( \frac{y_{\text{max}}}{M} \right)^2 M$$

$$= \frac{1}{2} \omega_n^2 \frac{m}{l} \left( \frac{33}{140} \right) + \frac{1}{2} \omega_n^2 \frac{Y_0^2}{M} \tag{E_3}$$

$$\therefore \quad m_{\text{eff1}} = \frac{33}{140} m = 0.2357 \text{ m}$$
(b) Let \( Y(x) = a_1 + a_2 x + a_3 x^2 + a_4 x^3 \)
\[ Y(0) = 0, \quad \frac{dY}{dx}(0) = 0, \quad Y(l) = Y_0, \quad \frac{d^2Y}{dx^2}(l) = 0 \]
This leads to \( Y(x) = \frac{3Y_0}{l^2} x^2 - \frac{2Y_0}{l^3} x^3 \) \hfill (E4)
\[ y(x,t) = Y_0 \left( \frac{3}{l^2} \frac{x^2}{l^2} - \frac{2}{l^3} \right) \cos(\omega_n t - \phi) \]
Maximum strain energy \( U_{strain} = \frac{1}{2} EI \int_0^l \left( \frac{d^2y}{dx^2} \right)^2 dx \) \hfill (E5)
\[ = \frac{6EIY_0^2}{l^3} \]
Max. kinetic energy \( U_{kinetic} = \frac{1}{2} M \omega_n^2 Y_0^2 + \frac{1}{2} (\frac{m}{l}) Y_0^2 \omega_n^2 \int_0^l \left( \frac{3}{l^2} \frac{x^2}{l^2} - \frac{2}{l^3} \right)^2 dx \) \hfill (E6)
\[ = \frac{1}{2} \omega_n^2 Y_0^2 \left( M + \frac{13}{35} m \right) \]
\[ \therefore m_{eff} = \frac{13}{35} m = 0.3714 m \]

2.39 Stiffnesses of segments:
\[
A_1 = \frac{\pi}{4} (D_1^2 - d_1^2) = \frac{\pi}{4} (2^2 - 1.75^2) = 0.7363 \text{ in}^2
\]
\[
k_1 = \frac{A_1E_1}{L_1} = \frac{(0.7363)(10^7)}{12} = 61.3583 \times 10^4 \text{ lb/in}
\]
\[
A_2 = \frac{\pi}{4} (D_2^2 - d_2^2) = \frac{\pi}{4} (1.5^2 - 1.25^2) = 0.5400 \text{ in}^2
\]
\[
k_2 = \frac{A_2E_2}{L_2} = \frac{(0.5400)(10^7)}{10} = 54.0 \times 10^4 \text{ lb/in}
\]
\[
A_3 = \frac{\pi}{4} (D_3^2 - d_3^2) = \frac{\pi}{4} (1^2 - 0.75^2) = 0.3436 \text{ in}^2
\]
\[
k_3 = \frac{A_3E_3}{L_3} = \frac{(0.3436)(10^7)}{8} = 42.9516 \times 10^4 \text{ lb/in}
\]
Equivalent stiffness (springs in series):
\[
\frac{1}{k_{eq}} = \frac{1}{k_1} + \frac{1}{k_2} + \frac{1}{k_3}
\]
\[= 0.0162977 \times 10^{-4} + 0.0185185 \times 10^{-4} + 0.0232820 \times 10^{-4} = 0.0580982 \times 10^{-4}
\]
or \( k_{eq} = 17.2122 \times 10^4 \text{ lb/in} \)

Natural frequency:
\[
\omega_n = \sqrt{\frac{k_{eq}}{m}} = \sqrt{\frac{k_{eq} g}{W}} = \sqrt{\frac{17.2122 \times 10^4 \times (386.4)}{10}} = 2578.9157 \text{ rad/sec}
\]
\[ 2.40 \]
\[ \frac{k}{l} \]
\[ \frac{k_1}{l} = \frac{k_2}{l} = \frac{k_3}{l} = \frac{1}{k_{total}} = \frac{1}{k_1} + \frac{1}{k_2} + \frac{1}{k_3} = \frac{1}{k} \]
\[ k_{total} = \frac{k_1}{k} \equiv k ; \; k_1 = 2k \]
\[ \sqrt{\frac{m}{k}} = \frac{1}{2\pi} \]
\[ \tau_n = 2\pi \sqrt{\frac{m}{ke_0}} \]
\[ 0.5 = 2\pi \sqrt{\frac{m}{4k}} \]
\[ k_2 = 4k \]
\[ k_3 = \frac{4}{3} k \]
\[ \tau_n = 2\pi \sqrt{\frac{m}{ke_0}} \quad \text{where} \quad ke_0 = 4k + \frac{4}{3} k = \frac{16}{3} k \]
\[ \therefore \tau_n = 2\pi \sqrt{\frac{m}{16k}} = \frac{2\pi \sqrt{3}}{4} \sqrt{\frac{m}{k}} = \frac{2\pi}{(2\pi)} \left( \frac{1}{2\pi} \right) = 0.4330 \text{ sec} \]

\[ 2.41 \]
Let \( \mu = \) coefficient of friction
\( x = \) displacement of c.g. of block
\( F_1, F_2 = \) net reactions between roller and block on left and right sides.
Reactions at left and right due to static load \( W \) are
\( W(c-x)/2c \) and \( W(c+x)/2c \), respectively.
\( M = \) moment about \( G \) due to motion of block = \( (\mu F_2 - \mu F_1)\alpha \)
Reactions at left and right to balance \( M = \frac{M}{2c} = \frac{\mu a}{2c} (F_2 - F_1) \)
\( F_1 = \frac{W(c-x)}{2c} - \frac{\mu a}{2c} (F_2 - F_1) \); \( F_2 = \frac{W(c+x)}{2c} + \frac{\mu a}{2c} (F_2 - F_1) \)
Subtraction gives \( F_2 - F_1 = \frac{W\alpha}{c} (\frac{c}{c-\mu a}) = \frac{W\alpha}{c-\mu a} \)
\( \therefore F_2 - F_1 = \frac{W\alpha}{c} (\frac{c}{c-\mu a}) = \frac{W\alpha}{c-\mu a} \)
Restoring force = \( \mu (F_2 - F_1) = \left( \frac{\mu W\alpha}{c-\mu a} \right) \)
Equation of motion:
\[ \frac{W}{g} \ddot{x} + \frac{\mu W}{(c-\mu a)} x = 0 \]
\[ \omega_n = \omega = \sqrt{\frac{\mu W g}{W(c-\mu a)} = \sqrt{\frac{\mu g}{c-\mu a}}} \]
Solving this, we get \( \omega = \left[ c \omega^2/(g + a \omega^2) \right] \)
From problem 2.41, 
Restoring force without springs = $\mu (F_2 - F_1) = \frac{\mu W x}{c - \mu a}$

Spring restoring force = $2kx$

Total restoring force = $\frac{\mu W x}{c - \mu a} + 2kx$

Equation of motion: $\frac{W}{g} \ddot{x} + \left(\frac{\mu W}{c - \mu a} + 2k\right)x = 0$

$\omega_n = \omega = \left\{\frac{\left[\mu W + 2k(c - \mu a)\right]g}{(c - \mu a)W}\right\}^{1/2}$

Solution of this equation gives

$\mu = \left(\frac{\omega^2 W c - 2k g c}{Wg + W \omega^2 a - 2k g a}\right)$

2.43 (a) Natural frequency of vibration of electromagnet (without the automobile):

$\omega_n = \sqrt{\frac{k}{M}} = \sqrt{\frac{10000.0 \times 386.4}{3000.0}} = 35.8887 \text{ rad/sec}$

(b) When the automobile is dropped, the electromagnet moves up by a distance ($x_0$) from its static equilibrium position.

$x_0 = \text{ static deflection (elongation of cable) under the weight of automobile} = \frac{W_{\text{auto}}}{k} = \frac{2000}{10000} = 0.2 \text{ in}$

$x_0 = 0$

Resultant motion of electromagnet ($+x$ upwards):

$x(t) = A_0 \sin (\omega_n t + \phi_0)$

where

$A_0 = \left\{x_0^2 + \left(\frac{x_0}{\omega_n}\right)^2\right\}^{1/2} = x_0 = 0.2$

and $\phi_0 = \tan^{-1}\left(\frac{x_0 \omega_n}{x_0}\right) = \tan^{-1}(\infty) = 90^\circ$

Hence $x(t) = 0.2 \sin (35.8887 t + 90^\circ) = 0.2 \cos 35.8887 t$

(c) Maximum $x(t)$:

$x(t) \mid_{\text{max}} = A_0 = 0.2 \text{ in}$

Maximum tension in cable during motion = $k \ x(t) \mid_{\text{max}} + \text{Weigh of electromagnet}$

$= 10000 \ (0.2) + 3000 = 5000 \text{ lb.}$
(a) Newton's second law of motion:
\[ F(t) = -k_1 x - k_2 x = m \ddot{x} \] or 
\[ m \ddot{x} + (k_1 + k_2) x = 0 \]

(b) D'Alembert's principle:
\[ F(t) - m \ddot{x} = 0 \] or 
\[ -k_1 x - k_2 x - m \ddot{x} = 0 \]
Thus 
\[ m \ddot{x} + (k_1 + k_2) x = 0 \]

(c) Principle of virtual work:
When mass \( m \) is given a virtual displacement \( \delta x \),
Virtual work done by the spring forces \( = -(k_1 + k_2) x \delta x \)
Virtual work done by the inertia force \( = -m \ddot{x} \delta x \)
According to the principle of virtual work, the total virtual work done by all forces must be equal to zero:
\[ -m \ddot{x} \delta x - (k_1 + k_2) x \delta x = 0 \] or 
\[ m \ddot{x} + (k_1 + k_2) x = 0 \]

(d) Principle of conservation of energy:
\[ T = \text{kinetic energy} = \frac{1}{2} m \dot{x}^2 \]
\[ U = \text{strain energy} = \text{potential energy} = \frac{1}{2} k_1 x^2 + \frac{1}{2} k_2 x^2 \]
\[ T + U = \frac{1}{2} m \dot{x}^2 + \frac{1}{2} (k_1 + k_2) x^2 = \text{constant} \]
\[ \frac{d}{dt} (T + U) = 0 \] or 
\[ m \ddot{x} + (k_1 + k_2) x = 0 \]
Equation of motion:

\[
\begin{align*}
\text{Mass } m: \quad m \ddot{x} &= T - m \ddot{x} \\
\text{Pulley } J_0: \quad J_0 \ddot{\theta} &= T r - k 4 r (\theta + \theta_0) 4 r
\end{align*}
\]

(1) \hspace{1cm} (2)

where \( \theta_0 \) = angular deflection of the pulley under the weight, \( mg \), given by:

\[
m g r = k (4 r \theta_0) 4 r \quad \text{or} \quad \theta_0 = \frac{m g}{16 r k}
\]

(3)

Substituting Eqs. (1) and (3) into (2), we obtain

\[
J_0 \ddot{\theta} = (m g - m \ddot{x}) r - k 16 r^2 (\theta + \frac{m g}{16 r k})
\]

(4)

Using \( x = r \theta \) and \( \ddot{x} = r \ddot{\theta} \), Eq. (4) becomes

\[
(J_0 + m r^2) \ddot{\theta} + (16 r^2 k) \theta = 0
\]

2.46 Consider the springs connected to the pulleys (by rope) to be in series. Then

\[
\frac{1}{k_{eq}} = \frac{1}{k} + \frac{1}{5k} \quad \text{or} \quad k_{eq} = \frac{5}{6} k
\]

Let the displacement of mass \( m \) be \( x \).

Then the extension of the rope (springs connected to the pulleys) = \( 2 x \). From the free body diagram, the equation of motion of mass \( m \):

\[
m \dddot{x} + 2 k x + k_{eq} (2 x) = 0
\]

or \( m \dddot{x} + \frac{11}{3} k x = 0 \)

2.47 \( T \) = kinetic energy = \( T_{mass} + T_{pulley} \)

\[
T = \frac{1}{2} m \dddot{x}^2 + \frac{1}{2} J_0 \ddot{\theta}^2 = \frac{1}{2} (m r^2 + J_0) \ddot{\theta}^2
\]

\[
U = \text{potential energy} = \frac{1}{2} k x^2 = \frac{1}{2} k (4 r \theta)^2 = \frac{1}{2} k (16 r^2) \theta^2
\]

Using \( \frac{d}{dt} (T + U) = 0 \) gives

\[
(m r^2 + J_0) \dddot{\theta} + (16 r^2 k) \theta = 0
\]
\[ 2.48 \]

\[ T = \text{kinetic energy} = \frac{1}{2} m \dot{x}^2 + \frac{1}{2} J_0 \dot{\theta}^2 \]

\[ U = \text{potential energy} = \frac{1}{2} k x_s^2 \]

where \( \theta = \frac{x}{r} \), \( x_s \) = extension of spring = 4 r \( \theta = 4 x \). Hence

\[ T = \frac{1}{2} \left( m + \frac{J_0}{r^2} \right) \dot{x}^2 ; \quad U = \frac{1}{2} (16 k) x^2 \]

Using the relation \( \frac{d}{dt} (T + U) = 0 \), we obtain the equation of motion of the system as:

\[ (m + \frac{J_0}{r^2}) \ddot{x} + 16 k x = 0 \]
2.49 (a) Stiffness of the cantilever beam of length \( l \) at location of the mass:

\[
k_b = \frac{3EI}{l^3}
\]  

(E1)

Since any transverse force \( F \) applied to the mass \( m \) is felt by each of the three springs \( k_1, k_2 \) and \( k_3 \), all the springs \( (k_1, k_2, k_3 \) and \( k_b) \) can be considered to be in series. The equivalent spring constant \( k_{eq} \) of the system is given by

\[
\frac{1}{k_{eq}} = \frac{1}{k_1} + \frac{1}{k_2} + \frac{1}{k_3} + \frac{1}{k_b}
\]

\[
k_{eq} = \frac{k_2 k_3 k_b + k_1 k_3 k_b + k_1 k_2 k_b + k_1 k_2 k_3}{k_1 k_2 k_3 k_b}
\]  

(E2)

or

\[
k_{eq} = \frac{k_2 k_3 k_b + k_1 k_3 k_b + k_1 k_2 k_b + k_1 k_2 k_3}{k_1 k_2 k_3 k_b}
\]  

(E3)

(b) Natural frequency of vibration of the system is given by

\[
\omega_n = \sqrt{\frac{k_{eq}}{m}}
\]

where \( k_{eq} \) is given by Eq. \( E3 \).
To find the stiffness of the given system at point C (i.e., deflection produced at C by a force $F$ applied at C), we use the following 6-step process:

**Step 1**: Consider a cantilever beam with force applied at C (without support at B) and find deflections at B and C:

$$
s(x) = \frac{F x^2}{6EI} (3l - x) \quad (1)
$$

$$
s_B = (\text{with } x = a) = \frac{F a^2}{6EI} (3l - a) = \frac{F(0.8)^2(3 - 0.8)}{6EI}
$$

$$
= 0.2347 \frac{F}{EI} \quad (2)
$$

$$
s_C = (\text{with } x = l) = \frac{Fl^2(2l)}{6EI} = \frac{F(1^2)(2)}{6EI} = 0.3333 \frac{F}{EI} \quad (3)
$$

\text{L} \rightarrow \delta_1
Step 2: Consider a cantilever beam with force $P$ applied at B (in upward direction) and find deflections at B and C:

$$\delta(x) = \frac{P}{6EI} \left(3a - x\right)$$  \hspace{1cm} (4)

$$\delta_B = \frac{P(a^2)}{6EI} = \frac{2P}{6EI} = \frac{0.17067P}{EI} \hspace{1cm} (5)$$

$$\delta_{BC}(x) = \frac{Pa^2}{6EI} \left(3a - x\right)$$  \hspace{1cm} (6)

$$\delta_C = \left(\text{at } x = L\right) = \frac{P(a^2)}{6EI} \left(3L - a\right)$$

$$= \frac{P}{6EI} \left(0.8^3 \cdot 3 - 0.8\right) = \frac{0.2347P}{EI} \hspace{1cm} (7)$$

Step 3: Find the value of $P$ needed to cause

$$\delta_B \quad \text{(in Eq. (5))} = -\delta_C \quad \text{(in Eq. (2))}$$

i.e.,

$$\frac{0.17067P}{EI} = -\frac{0.2347P}{EI}$$

i.e.,

$$P = \text{new} = -1.3749 \text{ F}$$  \hspace{1cm} (8)

Step 4: Find the value of upward deflection caused by $P_{\text{new}}$ at C (by using Eq. (8) in Eq. (7)):

$$\delta_C = \frac{0.2347(-1.3749F)}{EI} = -\frac{0.3227F}{EI} \hspace{1cm} (9)$$

$$\implies \delta_2$$

Step 5: Superpose the deflections of Step 2 with $P_{\text{new}}$ (in place of $P$) to obtain zero deflection at B and $$\left\{ \frac{0.3333F}{EI} - \frac{0.3227F}{EI} \quad \text{(Eq. (3) + Eq. (9))} = \frac{0.0106F}{EI} = \delta_1 - \delta_2 \right\}$$

Step 6: Thus we find net deflection of point C ($\delta_{cn}$) as

$$\delta_{cn} = \delta_1 - \delta_2 = \frac{0.0106F}{EI}$$

2-30
The stiffness of the beam (given system) due to force \( F \) applied at \( C \) is

\[
  k_c = \frac{F}{\delta_{cn}} = \frac{EI}{0.0106} = 94.3396 \text{ EI}
\]

Here \( E = 207 \times 10^9 \text{ Pa} \) and \( I = \frac{1}{12} (0.05)(0.05)^3 \)
\[= 52.1 \times 10^{-8} \text{ m}^4; \quad EI = 107,847\]

Natural frequency of the system:

\[
  \omega_n = \sqrt{\frac{k_c}{m}} = \sqrt{\frac{94.3396 \times (107,847)}{50}}
\]

\[= 451.0930 \text{ rad/s}\]
Deflection \( \delta \) due to \( F \):

\[
\delta_{AB}(x) = -\frac{F b x}{6EI} \left( \frac{x^2 + b^2 - l^2}{l} \right)
\]

At point B:

\[
\delta_B = -\frac{F (0.2)(0.8)}{6EI} \left( 0.64 + 0.04 - 1.0 \right)
\]

\[
= \frac{0.008533F}{EI}
\]

Stiffness of beam at B:

\[
k_B = \frac{F}{\delta_B} = \frac{EI}{0.008533F}
\]

\[\text{i.e.,} \]

\[k_B = 117.1875 \frac{EI}{F}
\]

Here \( I = \frac{1}{12} (0.05)(0.05)^3 = 52.1 \times 10^{-8} \text{ m}^4 \)

and \( E = 207 \times 10^9 \text{ Pa} \)

\[EI = (207 \times 10^9)(52.1 \times 10^{-8}) = 107,847.0 \text{ N}\cdot\text{m}^2
\]

\[k_B = 117.1875 \left( 107,847.0 \right) = 12,638,820.3 \frac{N}{m}
\]

Natural frequency of the system:

\[
\omega_n = \sqrt{\frac{k_B}{m}} = \sqrt{\frac{12,638,820.3}{50}}
\]

\[= 502.7588 \text{ rad/s}
\]
\[ a = 0.8 \text{ m} \]
\[ b = 0.2 \text{ m} \]

\[ y_{AB} = \frac{F b^2 x^2}{6EI} \left\{ 3a \cdot l - x(3a + b) \right\} \]

\[ y_B = \frac{F(0.2^2)(0.8^2)}{6EI} \left\{ 3(0.8)(1.0) - 0.8 \left( 3 \times 0.8 + 0.2 \right) \right\} \]

\[ = \frac{F(0.0256)(0.32)}{6EI} = \frac{0.001365F}{EI} \]

\[ k_B = \frac{F}{y_B} = \frac{EI}{0.001365} = 732.4219 \text{ EI} \]

\[ m = 50 \text{ kg} \]

\[ EI = (207 \times 10^9) \left( \frac{1}{12} (0.05)(0.05)^3 \right) \]

\[ = 107847.0 \text{ N-m}^2 \]

\[ k_B = 732.4219 \left( 107847.0 \right) = 78989504.65 \text{ N/m} \]

\[ \omega_n = \sqrt{\frac{k_B}{m}} = \sqrt{\frac{78989504.65}{50}} \]

\[ = 1256.8970 \text{ rad/s} \]
2.53

\[ a = 0.8 \text{ m} \]
\[ b = 0.2 \text{ m} \]
\[ l = 1.0 \text{ m} \]

\[ y_{AB} = \frac{F x^2}{6EI} (3a-x) \]

\[ y_B = y_{AB} \big|_{x=0.8} = \frac{F (0.8^2)}{6EI} (3 \times 0.8 - 0.8) \]

\[ = \frac{0.17067 F}{EI} \]

\[ k_B = \frac{F}{y_B} = \frac{EI}{0.17067} = 5.85937 \text{ EI} \]

\[ m = 50 \text{ kg} \]

\[ EI = (207 \times 10^9) \left\{ \frac{1}{12} (0.05) (0.05)^3 \right\} \]

\[ = 107,847.0 \]

\[ k_B = 5.85937 \times (107,847.0) = 631,915.4764 \text{ N/m} \]

\[ \omega_n = \sqrt{\frac{k_B}{m}} = \sqrt{\frac{631,915.4764}{50}} \]

\[ = 112.4202 \text{ rad/s} \]

2-34
Beam on simple supports with overhang:

\[ y_C = \frac{F a^2}{3EI} (l + a) \]

\[ = \frac{F (0.2^2) (0.8 + 0.2)}{3EI} = \frac{0.04 F}{3EI} \]

\[ k_C = \frac{F}{y_C} = \frac{3EI}{0.04} = 75EI \]

\[ = 75 \left( \frac{207 \times 10^9}{12 (0.05) (0.05)^3} \right) \]

\[ = 75 \left( 107,847.6 \right) = 808,852.5 \text{ N/m} \]

\[ \omega_n = \sqrt{\frac{k_c}{m}} = \sqrt{\frac{808,852.5}{50}} = 402.2070 \text{ rad/s} \]
Equivalent stiffness of spring \( k \) at location of mass \( m \):

Assume the beam as a rigid bar ABC hinged at point A to find the equivalent stiffness of spring \( k \) at point B (\( k_B \)). Let the equivalent spring constant of \( k \) when located at B be \( k_B \).

Then we equate the moments created at point A by the spring force due to \( k \) at C and the spring force due to \( k_B \) at B:

\[ k_C \delta_C l = k_B \delta_B a \]

i.e., \( k_B = \frac{k_C \delta_C l}{\delta_B} = k \frac{\theta l}{a} \frac{l}{a} = \frac{k l^2}{a^2} \]

\[ = 10000 \left( \frac{1^2}{0.8^2} \right) = 15625 \text{ N/m} \]

Spring constant of the beam at location of mass \( m \):

For simplicity, we assume that the spring at C acts as a simple support. This permits the computation of 2-36
the equivalent spring constant of the beam ABC subjected to a force F at B.

\[
K_{beam,B} = \frac{F}{\delta_{beam,B}}
\]

\[
\delta_{AB}(x) = \frac{Fb^2x^2}{6EI} \left\{3aL - x(3a+b)\right\}
\]

\[
\delta_{beam,B} = \frac{F(0.2^2)(0.8^2)}{6EI(1^3)} \left\{3(0.8)(1.0) - 0.8(3\times0.8+0.2)\right\}
\]

\[
= 0.001365 \frac{F}{EI}
\]

\[
K_{beam,B} = \frac{F}{\delta_{beam,B}} = \frac{EI}{0.001365} = 732.4219 EI
\]

\[
EI = (207 \times 10^9) \frac{1}{12} (0.05)(0.05)^3 = 207 \times 10^9 \times 52.1 \times 10^{-8}
\]

\[
= 107,847.0
\]

\[
K_{beam,B} = 732.4219 (107,847.0) = 78,989.5 \times 10^6 N/m
\]

Next, we consider the two springs \(K_B\) and \(K_{beam,B}\) to be parallel so that the equivalent spring constant at B, \(K_{eq,B}\), is given by

\[
K_{eq,B} = K_B + K_{beam,B} = 0.01562 \times 10^6 + 78.9895 \times 10^6
\]

2-37
\[ k_{eq} B = 79.00512 \times 10^6 \text{ N/m} \]

Natural frequency of vibration of the system:

\[ \omega_n = \sqrt{\frac{k_{eq} B}{m}} = \sqrt{\frac{79.00512 \times 10^6}{50}} \]

\[ = 1580.1024 \text{ rad/s} \]
simply supported beam with overhang:

\[ \delta_C = \frac{F b^2 (a+b)}{3EI} \]

\[ k_b = \kappa_\text{beam at } C = \frac{F}{\delta_C} = \frac{3EI}{b^2 (a+b)} \]

\[ E = 207 \times 10^9 \text{ Pa} \]

\[ I = \frac{1}{12} (0.05)(0.05)^3 = 52.1 \times 10^{-8} \text{ m}^4 \]

\[ k_b = \frac{3(207 \times 10^9)(52.1 \times 10^{-8})}{(0.04)(1.0)} \]

\[ = 8,088,525.0 \text{ N/m} \]

\[ \kappa_{\text{eq } C} = \text{equivalent stiffness constant at } C \]

\[ = k_b + k = 8,088,525.0 + 10,000.0 \]

\[ = 8,098,525.0 \text{ N/m} \]

Natural frequency of vibration of the system:

\[ \omega_n = \sqrt{\frac{\kappa_{\text{eq } C}}{m}} = \sqrt{\frac{8,098,525.0}{50}} \]

\[ = 402.4556 \text{ rad/s} \]
For a cantilever beam,

\[ k_b = k_{\text{beam at } C} = \frac{3EI}{l^3} \]

\[ = 3 \left( 207 \times 10^9 \right) \frac{1}{12} (0.05)(0.05)^3 \]

\[ = 323,541.0 \text{ N/m} \]

\[ k_{eq C} = \text{equivalent spring constant at } C \]

\[ = k_b + k = 323,541.0 + 10,000.0 \]

\[ = 333,541.0 \text{ N/m} \]

Natural frequency of vibration of the system:

\[ \omega_n = \sqrt{\frac{k_{eq C}}{m}} = \sqrt{\frac{333,541.0}{50}} \]

\[ = 81.6751 \text{ rad/s} \]
Assume the beam as a rigid bar ABC hinged at A to find the equivalent stiffness of spring $k$ at point C ($k_C$). We equate the moments created at point A by the spring force due to $k$ at B and the spring force due to $k_C$ at C:

$$k_C \delta_C \ell = k \delta_B \ell$$

i.e.,

$$k_C = \frac{k \delta_B \ell}{\delta_C} = \frac{k \theta \alpha \alpha}{\theta \ell \ell} = \frac{k \alpha^2}{\ell^2}$$

$$= 10000 \left(\frac{0.64}{1^2}\right) = 6400 \text{ N/m}$$

$$k_b = k_{\text{beam}} = \text{stiffness constant of the beam at location of mass } m$$

$$= \frac{3EI}{\ell^3} = \frac{3 \left(207 \times 10^9\right) \left\{ \frac{1}{12} \left(0.05\right) \left(0.05\right)^3 \right\}}{\ell^3}$$

2-41
i.e., \[ k_b = 323,541.0 \text{ N/m} \]

Equivalent spring constant at location of mass (m):

\[ k_{eq} = k_b + k_C \]

\[ = 323,541.0 + 6,400.0 = 329,941.0 \text{ N/m} \]

Natural frequency of vibration of the system:

\[ \omega_n = \sqrt{\frac{k_{eq}}{m}} = \sqrt{\frac{329,941.0}{50}} \]

\[ = 81.2331 \text{ rad/s} \]
\[ x(t) = A \cos (\omega_n t - \phi) \]  
\[ k = 2000 \text{ N/m}, \; m = 5 \text{ kg} \]
\[ \omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{2000}{5}} = 20 \text{ rad/s} \]
\[ A = \left\{ x_0^2 + \left( \frac{\dot{x}_0}{\omega_n} \right)^2 \right\}^{\frac{1}{2}}, \; \phi = \tan^{-1} \left( \frac{\ddot{x}_0}{x_0 \omega_n} \right) \]

(a) \[ x_0 = 20 \text{ mm}, \; \dot{x}_0 = 200 \text{ mm/s} \]
\[ A = \left\{ (20)^2 + \left( \frac{200}{20} \right)^2 \right\}^{\frac{1}{2}} = 22.3607 \text{ mm} \]
\[ \phi = \tan^{-1} \left( \frac{200}{20(20)} \right) = \tan^{-1} (0.5) \]
\[ = 26.5650^\circ \text{ or } 0.4636 \text{ rad} \]

Since both \( x_0 \) and \( \dot{x}_0 \) are positive, \( \phi \) will lie in the first quadrant. Thus the response of the system is given by Eq. (1):
\[ x(t) = 22.3607 \cos (20t - 0.4636) \text{ mm} \]

(b) \[ x_0 = -20 \text{ mm}, \; \dot{x}_0 = 200 \text{ mm/s} \]
\[ A = \left\{ (-20)^2 + \left( \frac{200}{20} \right)^2 \right\}^{\frac{1}{2}} = 22.3607 \text{ mm} \]
\[ \phi = \tan^{-1} \left( \frac{200}{(-20)(20)} \right) = \tan^{-1} (-0.5) \]
\[ = -26.5650^\circ \text{ (or } -0.4636 \text{ rad)} \text{ or } 153.4349^\circ \text{ (or } 2.6780 \text{ rad)} \]

Since \( x_0 \) is negative, \( \phi \) lies in the second quadrant. Thus the response of the system is:
\[ x(t) = 22.3607 \cos (20t - 2.6780) \text{ mm} \]
(c) $x_0 = 20 \text{ mm}, \quad \dot{x}_0 = -200 \text{ mm/s}$

$$A = \left\{ (20)^2 + \left( -\frac{200}{20} \right)^2 \right\}^{\frac{1}{2}} = 22.3607 \text{ mm}$$

$$\phi = \tan^{-1}\left( \frac{-200}{20(20)} \right) = \tan^{-1}(-0.5)$$

$$= -26.5650^\circ \text{ (or } -0.4636 \text{ rad}) \text{ or } 333.4350^\circ \text{ (or } 5.8196 \text{ rad})$$

Since $\dot{x}_0$ is negative, $\phi$ lies in the fourth quadrant. Thus the response of the system is given by

$$x(t) = 22.3607 \cos(20t + 0.4636) \text{ mm}$$

or

$$22.3607 \cos(20t - 5.8196) \text{ mm}$$

(d) $x_0 = -20 \text{ mm}, \quad \dot{x}_0 = -200 \text{ mm/s}$

$$A = \left\{ (-20)^2 + \left( -\frac{200}{20} \right)^2 \right\}^{\frac{1}{2}} = 22.3607 \text{ mm}$$

$$\phi = \tan^{-1}\left( \frac{-200}{-20(20)} \right) = \tan^{-1}(0.5)$$

$$= 26.5650^\circ \text{ (or } 0.4636 \text{ rad})$$

or

$$206.5650^\circ \text{ (or } 3.5952 \text{ rad})$$

Since both $x_0$ and $\dot{x}_0$ are negative, $\phi$ will be in the third quadrant. Hence the response of the system will be

$$x(t) = 22.3607 \cos(20t - 3.5952) \text{ mm}$$
\[ x(t) = A \cos(\omega_n t - \phi) \] (1)

with
\[ A = \sqrt{\dot{x}_0^2 + \left(\frac{\ddot{x}_0}{\omega_n}\right)^2}, \quad \phi = \tan^{-1}\left(\frac{\dot{x}_0}{x_0 \omega_n}\right) \]

\[ m = 10 \text{ kg}, \quad k = 1000 \text{ N/m} \]
\[ \omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{1000}{10}} = 10 \text{ rad/s} \]

(a) \[ x_0 = 100 \text{ mm}, \quad \dot{x}_0 = 100 \text{ mm/s} \]
\[ A = \sqrt{(10)^2 + \left(\frac{100}{10}\right)^2} = \sqrt{100 + 100} = 14.1421 \text{ mm} \]
\[ \phi = \tan^{-1}\left(\frac{100}{10}\right) = \tan^{-1}(10) = 45^\circ \text{ or } 0.7854 \text{ rad} \]

Since both \( x_0 \) and \( \dot{x}_0 \) are positive, \( \phi \) will be in the first quadrant. Hence the response of the system is given by Eq. (1):
\[ x(t) = 14.1421 \cos(10t - 0.7854) \text{ mm} \]

(b) \[ x_0 = -100 \text{ mm}, \quad \dot{x}_0 = 100 \text{ mm/s} \]
\[ A = \sqrt{(-10)^2 + \left(\frac{100}{10}\right)^2} = \sqrt{100 + 100} = 14.1421 \text{ mm} \]
\[ \phi = \tan^{-1}\left(\frac{100}{-10}\right) = \tan^{-1}(-10) = -45^\circ \text{ or } 135^\circ \]
\[ \text{ or } (-0.7854 \text{ rad or } 2.3562 \text{ rad}) \]

since \( x_0 \) is negative, \( \phi \) lies in the second quadrant. Thus the response of the system is given by
\[ x(t) = 14.1421 \cos(10t - 2.3562) \text{ mm} \]
(c) \( x_0 = 10 \text{ mm}, \quad \dot{x}_0 = -100 \text{ mm/s} \)

\[
A = \left\{ \left(10\right)^2 + \left(-\frac{100}{10}\right)^2 \right\}^{\frac{1}{2}} = 14.1421 \text{ mm}
\]

\[
\phi = \tan^{-1} \left( -\frac{100}{10} \right) = \tan^{-1} (-1) = -45^\circ \text{ or } 315^\circ \text{ (or } -0.7854 \text{ rad or } 5.4978 \text{ rad})
\]

Since \( x_0 \) is positive and \( \dot{x}_0 \) is negative, \( \phi \) lies in the fourth quadrant. Hence the response of the system is given by

\[
x(t) = 14.1421 \cos (10t - 5.4978) \text{ mm}
\]

(d) \( x_0 = -10 \text{ mm}, \quad \dot{x}_0 = -100 \text{ mm/s} \)

\[
A = \left\{ \left(-10\right)^2 + \left(-\frac{100}{10}\right)^2 \right\}^{\frac{1}{2}} = 14.1421 \text{ mm}
\]

\[
\phi = \tan^{-1} \left( -\frac{100}{-10} \right) = \tan^{-1} (1) = 45^\circ \text{ or } 225^\circ
\]

\[
= (0.7854 \text{ rad or } 2.3562 \text{ rad})
\]

Since both \( x_0 \) and \( \dot{x}_0 \) are negative, \( \phi \) lies in the third quadrant. Thus the response of the system will be

\[
x(t) = 14.1421 \cos (10t - 2.3562) \text{ mm}
\]
2.61 Computation of phase angle $\phi_o$ in Eq. (2.23):

**case (i):** $x_0$ and $\dot{x}_0$ are positive:

$$\tan \phi_o = \text{positive}; \text{ hence } \phi_o \text{ lies in first quadrant (as shown in Fig. A)}$$

**case (ii):** $x_0$ = positive, $\dot{x}_0$ (or $\ddot{x}_0/\omega_n$) = negative

$$\tan \phi_o = \text{negative}; \phi_o \text{ lies in second quadrant}$$

**case (iii):** $x_0$ = negative, $\dot{x}_0$ (or $\ddot{x}_0/\omega_n$) = negative

$$\tan \phi_o = \text{positive}; \phi_o \text{ lies in third quadrant}$$

**case (iv):** $x_0$ = negative, $\dot{x}_0$ (or $\ddot{x}_0/\omega_n$) = positive

$$\tan \phi_o = \text{negative}; \phi_o \text{ lies in fourth quadrant}$$

Figure A
m = 5 kg, \( k = 2000 \) N/m

\[ \omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{2000}{5}} = 20 \text{ rad/s} \]

(a) \( x_0 = 20 \) mm, \( \dot{x}_0 = 200 \) mm/s

Since \( x_0 \) and \( \dot{x}_0 \) are both positive, \( \phi_0 \) lies in the first quadrant (from solution of Problem 2.61):

\[ \phi_0 = \tan^{-1} \left( \frac{x_0 \omega_n}{\dot{x}_0} \right) = \tan^{-1} \left( \frac{20 (20)}{200} \right) = \tan^{-1} (2) \]

\[ = 63.4349^\circ \text{ or } 1.1071 \text{ rad} \]

Response given by Eq. (2.23):

\[ x(t) = A_0 \sin (\omega_n t + \phi_0) \]

with \( A_0 = \left\{ x_0^2 + \left( \frac{\dot{x}_0}{\omega_n} \right)^2 \right\}^{\frac{1}{2}} = \left\{ (20)^2 + \left( \frac{200}{20} \right)^2 \right\}^{\frac{1}{2}} \]

\[ = 22.3607 \text{ mm} \]

\[ x(t) = 22.3607 \sin (20t + 1.1071) \text{ mm} \]

(b) \( x_0 = -20 \) mm, \( \dot{x}_0 = 200 \) mm/s

Since \( x_0 \) is negative and \( \dot{x}_0 \) is positive, \( \phi_0 \) lies in the fourth quadrant (from Problem 2.61).

\[ \phi_0 = \tan^{-1} \left( \frac{x_0 \omega_n}{\dot{x}_0} \right) = \tan^{-1} \left( \frac{-20 (20)}{200} \right) \]

\[ = \tan^{-1} (2) = -63.4349^\circ \text{ or } -1.1071 \text{ rad} \text{ or } \]

\[ 296.5651^\circ \text{ or } 5.1760 \text{ rad} \]

\[ A_0 = \left\{ x_0^2 + \left( \frac{\dot{x}_0}{\omega_n} \right)^2 \right\}^{\frac{1}{2}} = \left\{ (-20)^2 + \left( \frac{200}{20} \right)^2 \right\}^{\frac{1}{2}} \]

\[ = 22.3607 \text{ mm} \]

\[ x(t) = 22.3607 \sin (20t + 5.1760) \text{ mm} \]
(c) $x_0 = 20 \text{ mm}, \quad \dot{x}_0 = -200 \text{ mm/s}$

$$\phi_0 = \tan^{-1}\left(\frac{x_0}{\dot{x}_0}\right) = \tan^{-1}\left(\frac{20}{-200}\right) = \tan^{-1}(-2)$$

$$= -63.4349^\circ \quad \text{(or } -1.1071 \text{ rad}) \quad \text{or}$$

$$116.5651^\circ \quad \text{(or } 2.0344 \text{ rad})$$

Since $x_0$ is positive and $\dot{x}_0$ is negative, $\phi_0$ lies in the second quadrant (from Problem 2.61).

$$A_0 = \left\{x_0^2 + \left(\frac{\dot{x}_0}{\omega_n}\right)^2\right\}^{\frac{1}{2}} = \left\{(20)^2 + \left(-\frac{200}{20}\right)^2\right\}^{\frac{1}{2}}$$

$$= 22.3607 \text{ mm}$$

$\therefore x(t) = 22.3607 \sin (20t + 2.0344) \text{ mm}$

(d) $x_0 = -20 \text{ mm}, \quad \dot{x}_0 = -200 \text{ mm/s}$

$$\phi_0 = \tan^{-1}\left(\frac{-20}{-200}\right) = \tan^{-1}(2) = 63.4349^\circ$$

or $1.1071 \text{ rad} \quad \text{(or } 2.4349^\circ \text{ or } 4.2487 \text{ rad})$

$$A_0 = \left\{(-20)^2 + \left(-\frac{200}{20}\right)^2\right\}^{\frac{1}{2}}$$

$$= 22.3607 \text{ mm}$$

$\therefore x(t) = 22.3607 \sin (20t + 4.2487) \text{ mm}$

(since $x_0$ and $\dot{x}_0$ are both negative, $\phi_0$ lies in the third quadrant, from solution of Problem 2.61).
\[ m = 10 \text{ kg, } k = 1000 \text{ N/m} \]

\[ \omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{1000}{10}} = 10 \text{ rad/s} \]

Solution (response) of the system is given by

\[ x(t) = A_0 \sin (\omega_n t + \phi_0) \text{ mm} \]

with

\[ A_0 = \left\{ \frac{x_0^2 + (\frac{x_0}{\omega_n})^2}{2} \right\}^{\frac{1}{2}} \text{ and } \phi_0 = \tan^{-1} \left( \frac{x_0}{\omega_n \dot{x}_0} \right) \]

(a) \( x_0 = 10 \text{ mm, } \dot{x}_0 = 100 \text{ mm/s} \)

\[ A_0 = \left\{ (10)^2 + (\frac{100}{10})^2 \right\}^{\frac{1}{2}} = \sqrt{200} = 14.1421 \text{ mm} \]

\[ \phi_0 = \tan^{-1} \left( \frac{10}{100} \right) = \tan^{-1} (1) = 45^\circ \text{ or } 0.7854 \text{ rad} \]

Because \( x_0 \) and \( \dot{x}_0 \) are both positive, \( \phi_0 \) lies in the first quadrant (from Problem 2.61).

\( \therefore x(t) = 14.1421 \sin (10t + 0.7854) \text{ mm} \)

(b) \( x_0 = -10 \text{ mm, } \dot{x}_0 = 100 \text{ mm/s} \)

\[ A_0 = \left\{ (-10)^2 + (\frac{100}{10})^2 \right\}^{\frac{1}{2}} = 14.1421 \text{ mm} \]

\[ \phi_0 = \tan^{-1} \left( \frac{-10}{100} \right) = \tan^{-1} (-1) = -45^\circ \text{ or } -0.7854 \text{ rad (or 315^\circ or 5.4978 rad)} \]

Since \( x_0 \) is negative and \( \dot{x}_0 \) is positive, \( \phi_0 \) lies in the fourth quadrant (from Problem 2.61).

\( \therefore x(t) = 14.1421 \sin (10t + 5.4978) \text{ mm} \)
(c) \( x_0 = 10 \text{ mm}, \quad \dot{x}_0 = -100 \text{ mm/s} \)

\[ A_0 = \sqrt{(10)^2 + \left(-\frac{100}{10}\right)^2} = 14.1421 \text{ mm} \]

\[ \phi_0 = \tan^{-1}\left(\frac{10}{-100}\right) = \tan^{-1}(-1) = 135^\circ \text{ or } 2.3562 \text{ rad} \]

Since \( x_0 \) is positive and \( \dot{x}_0 \) is negative, \( \phi_0 \) lies in the second quadrant (from Problem 2.61).

\[ \therefore x(t) = 14.1421 \sin (10t + 2.3562) \text{ mm} \]

(d) \( x_0 = -10 \text{ mm}, \quad \dot{x}_0 = -100 \text{ mm/s} \)

\[ A_0 = \sqrt{(-10)^2 + \left(-\frac{100}{10}\right)^2} = 14.1421 \text{ mm} \]

\[ \phi_0 = \tan^{-1}\left(\frac{-10}{-100}\right) = \tan^{-1}(1) = 225^\circ \text{ or } 3.9270 \text{ rad} \]

Since both \( x_0 \) and \( \dot{x}_0 \) are negative, \( \phi_0 \) lies in the third quadrant (from Problem 2.61).

\[ \therefore x(t) = 14.1421 \sin (10t + 3.9270) \text{ mm} \]
From Example 2.1, \( m = 1 \, \text{kg}, \, k = 2500 \, \text{N/m} \)

\[
\omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{2500}{1}} = 50 \, \text{rad/s}
\]

\( x_0 = -2 \, \text{mm}, \, \dot{x}_0 = 100 \, \text{mm/s} \)

Eq. (2.23) is:

\[
x(t) = A_0 \sin (\omega_n t + \phi_0)
\]

with

\[
A_0 = \left\{ x_0^2 + \left( \frac{\dot{x}_0}{\omega_n} \right)^2 \right\}^{\frac{1}{2}}
\]

and

\[
\phi_0 = \tan^{-1} \left( \frac{x_0 \omega_n}{\dot{x}_0} \right)
\]

For the given data,

\[
A_0 = \left\{ (-2)^2 + \left( \frac{100}{50} \right)^2 \right\}^{\frac{1}{2}} = 2.8284 \, \text{mm}
\]

\[
\phi_0 = \tan^{-1} \left( \frac{-2}{100} \right) = \tan^{-1} (-0.02) = -0.7854 \, \text{rad}
\]

or

\[315^\circ \text{ or } 5.14978 \, \text{rad}\]

since \( x_0 \) is negative and \( \dot{x}_0 \) is positive, \( \phi_0 \) lies in the fourth quadrant (from Problem 2.61).

\[\therefore \text{Response is given by}\]

\[
x(t) = 2.8284 \sin (50t + 5.14978) \, \text{mm}
\]
(a) The area moment of inertia of the solid circular cross-section of the tree \((I)\) is given by

\[ I = \frac{1}{64} \pi d^4 = \frac{1}{64} \pi (0.25)^4 = 0.000191748 \text{ m}^4 \]

The axial load acting on the top of the trunk is:

\[ F = m_c g = 100 \times (9.81) = 981 \text{ N} \]

Assuming the trunk as a fixed-free column under axial load, the buckling load can be determined as

\[ P_{cr} = \frac{\pi^2 EI}{\ell^2} = \frac{\pi^2}{4} \left(1.2 \times 10^9\right) \left(191.748 \times 10^{-6}\right) \left(10^2\right) \]

\[ = 5677.4573 \text{ N} \]

since the axial force due to the mass of the crown \((F)\) is smaller than the critical load, the tree trunk will not buckle.

(b) The spring constant of the trunk in sway (transverse) motion is given by (assuming the trunk as a fixed-free beam)

\[ k = \frac{3EI}{\ell^3} = \frac{3}{4} \left(1.2 \times 10^9\right) \left(191.748 \times 10^{-6}\right) \left(10^3\right) \]

\[ = 690.2928 \text{ N/m} \]

Natural frequency of vibration of the tree is given by

\[ \omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{690.2928}{100}} = 2.6273 \text{ rad/s} \]
(a) Mass of bird = \( m_b = 2 \text{ kg} \)

Mass of beam (branch) = \( m_{br} = \frac{\pi d^2}{4} l f \)

\[
m_{br} = \frac{\pi (0.1)^2}{4} (4)(700) = 21.9912 \text{ kg}
\]

\( M = \text{total mass at } B = \text{mass of bird + equivalent mass of beam (AB) at } B \)

\[
= 2 + 0.23 \times 21.9912 = 7.0580 \text{ kg}
\]

(From equation of motion, refer to its free end = 0.23 times its total mass)

\( \kappa = \text{stiffness of cantilever beam (branch) at end } B \)

\[
= \frac{3EI}{l^3} = \frac{3 \times (10 \times 10^9)}{1^3} \frac{\pi}{64} (0.1)^4
\]

\[
= 2301.0937 \text{ N/m}
\]

Thus the equation of motion of the bird, in free vibration, is given by

\[ M \ddot{x} + \kappa x = 0 \quad \text{(by assuming no damping)} \]

\[
i.e. \quad 7.0580 \ddot{x} + 2301.0937 x = 0
\]

(b) Natural frequency of vibration of the bird:

\[
\omega_n = \sqrt{\frac{\kappa}{M}} = \sqrt{\frac{2301.0937}{7.0580}} = 18.0562 \text{ rad/s}
\]
Given: mass of bird \((m) = 2 \text{ kg}\)

height of branch \((\text{length of cantilever beam}) = h = 2 \text{ m}\)

density of branch \(= \rho = 700 \text{ kg/m}^3\)

Young's modulus of branch \(= E = 10 \text{ GPa}\)

(a) Buckling load of a cantilever beam with axial force applied at free end is given by

\[ P_{cr} = \frac{1}{4} \frac{\pi^2 EI}{h^2} \tag{1} \]

Assuming the diameter of branch as \(d\), the area moment of inertia \((I)\) is given by

\[ I = \frac{\pi d^4}{32} \tag{2} \]

When critical load \((P_{cr})\) is set equal to the weight of bird,

\[ P_{cr} = mg = 2 \times (9.81) = 19.62 \text{ N} \tag{3} \]

Equating Eq. (3) to Eq. (1), we obtain

\[ 19.62 = \frac{1}{4} \frac{\pi^2 (10 \times 10^9)}{2^2} \left( \frac{\pi d^4}{64} \right) \]

\[ = 0.3028 \times 10^9 \text{ N} \]

\[ \therefore d^4 = \frac{19.62}{0.3028 \times 10^9} = 6.4735 \times 10^{-8} \]

\[ \therefore d = 1.5954 \times 10^{-2} = 0.015954 \text{ m} \]

\[ \therefore \text{Minimum diameter of the branch to avoid buckling under the weight of the bird (neglecting the weight of the branch) is} \]

\[ d = 1.595 \text{ cm} \]
(b) Natural frequency of vibration of the system in bending \((\omega_{n,b})\):

\[
\omega_{n,b} = \sqrt{\frac{k}{m}}
\]

where \(m = 2 \text{ kg}\) (neglecting mass of branch), and \(k = \text{bending stiffness of cantilever beam of length } h\):

\[
= \frac{3EI}{h^3} = 3 \left(10 \times 10^9\right) \left\{\frac{\pi}{64} \left(0.01595\right)^4\right\}
\]

\[
= 11.9137 \text{ N/m}
\]

Thus \(\omega_{n,b} = \sqrt{\frac{11.9137}{2}} = 2.4407 \text{ rad/s}\)

Natural frequency of vibration of the system in axial motion \((\omega_{n,a})\):

\[
\omega_{n,a} = \sqrt{\frac{k_a}{m}}
\]

where \(m = 2 \text{ kg}\) and

\[
k_a = \frac{AE}{L} = \frac{\pi}{4} \left(0.01595\right)^2 \left(10 \times 10^9\right)
\]

\[
= 0.9990 \times 10^6 \text{ N/m}
\]

Thus \(\omega_{n,a} = \sqrt{\frac{0.9990 \times 10^6}{2}} = 706.7531 \text{ rad/s}\)
2.68 \[ m = 2 \text{ kg}, \ k = 500 \text{ N/m}, \ x_o = 0.1 \text{ m}, \ \dot{x}_o = 5 \text{ m/s} \]
\[ \omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{500}{2}} = 15.8114 \text{ rad/s} \]

Displacement of mass (given by Eq. (2.21)):
\[ x(t) = A \cos(\omega_n t - \phi) \]
where
\[ A = \left[ x_o^2 + \left(\frac{\dot{x}_o}{\omega_n}\right)^2\right]^{\frac{1}{2}} = \left[ 0.1^2 + \left(\frac{5}{15.8114}\right)^2\right]^{\frac{1}{2}} = \sqrt{0.11} \]
\[ = 0.3317 \text{ m} \]
\[ \phi = \tan^{-1}\left(\frac{\dot{x}_o}{\omega_n x_o}\right) = \tan^{-1}\left(\frac{5}{15.8114 \times 0.1}\right) \]
\[ = \tan^{-1}(3.1623) = 72.4516^\circ \text{ or } 1.2645 \text{ rad} \]

(\( \phi \) will be in the first quadrant because both \( x_o \) and \( \dot{x}_o \) are positive)
\[ x(t) = 0.3317 \cos(15.8114 t - 1.2645) \text{ m} \]
\[ \dot{x}(t) = -5.2446 \sin(15.8114 t - 1.2645) \text{ m/s} \]
\[ \ddot{x}(t) = -82.9251 \cos(15.8114 t - 1.2645) \text{ m/s}^2 \]

Data: \( \omega_n = 10 \text{ rad/s}, \ x_o = 0.05 \text{ m}, \ \dot{x}_o = 1 \text{ m/s} \)

Response of undamped system:
\[ x(t) = x_o \cos \omega_n t + \frac{\dot{x}_o}{\omega_n} \sin \omega_n t \]
\[ = 0.05 \cos 10t + \left(\frac{1}{10}\right) \sin 10t \]
\[ x(t) = 0.05 \cos 10t + 0.1 \sin 10t \quad \text{m} \quad (E.1) \]
\[ \dot{x}(t) = -0.5 \sin 10t + \cos 10t \quad \text{m/s} \quad (E.2) \]
\[ \ddot{x}(t) = -5 \cos 10t - 10 \sin 10t \quad \text{m/s}^2 \quad (E.3) \]

Plotting of Eqs. (E.1) to (E.3):

```matlab
% Ex2_52.m
for i = 1: 1000
    t(i) = (i-1)*5/1000;
    x(i) = 0.05 * cos(10*t(i)) + 0.1*sin(10*t(i));
    dx(i) = -0.5*sin(10*t(i)) + cos(10*t(i));
    ddx(i) = -5*cos(10*t(i)) - 10*sin(10*t(i));
end
plot(t, x);
hold on;
plot(t, dx, '--');
plot(t, ddx, ':');
xlabel('t');
ylabel('x(t), dx(t), ddx(t)');
title('Solid line: x(t) Dashed line: dx(t) Dotted line: ddx(t)');
```

![Graph of x(t), dx(t), and ddx(t)](image-url)
2.70  Data: \( \omega_d = 2 \text{ rad/s}, \quad \xi = 0.1, \quad X_0 = 0.01 \text{ m}, \quad \phi_0 = 1 \text{ rad} \)

Initial conditions:

\[
\omega_d = \sqrt{1 - \xi^2} \quad \omega_n = \frac{\omega_d}{\sqrt{1 - \xi^2}} = \frac{2}{\sqrt{1 - 0.01}}
\]

(E.1)

Eqs. (2.73), (2.75): \( \tau = 2.0101 \text{ rad/s} \)

(E.2)

\[
\phi_0 = \tan^{-1} \left( -\frac{\dot{X}_0 + 0.20101 X_0}{\omega_d X_0} \right) = 1
\]

(E.3)

Eqs. (E.2) and (E.3) lead to:

\[
X_0^2 + \left( \frac{\dot{X}_0 + 0.20101 X_0}{2} \right) = 0.0001
\]

(E.4)

or

\[
- \left( \frac{\dot{X}_0 + 0.20101 X_0}{2 X_0} \right) = \tan 1 = 0.7854
\]

(E.5)

Substitution of Eq. (E.5) into (E.4) yields

\[
X_0 = 0.007864 \text{ m}
\]

(E.6)

Eqs. (E.6) and (E.5) give

\[
\dot{X}_0 = -0.013933 \text{ m/s}
\]

(E.7)

2.71  Without passengers,

\[
(\omega_n)_1 = \sqrt{\frac{k}{m}} = 20 \text{ rad/s} \quad \Rightarrow \quad k = 400 \text{ m}
\]

(E.1)

With passengers,

\[
(\omega_n)_2 = \sqrt{\frac{k}{m + 500}} = 17.32 \text{ rad/s}
\]

(E.2)

Squaring Eq. (E.2), we get

\[
\frac{k}{m + 500} = (17.32)^2 = 299.9824
\]

(E.3)

Using \( k = 400 \text{ m} \) in (E.3) gives

\[
m = 1499.6481 \text{ kg}
\]
\( 2.72 \)  
\[
\omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{3200}{2}} = 40 \text{ rad/s}
\]

\( x_0 = 0 \)

\[
X_0 = \sqrt{x_0^2 + \left( \frac{\dot{x}_0}{\omega_n} \right)^2} = 0.1
\]

i.e. \( \frac{\dot{x}_0}{\omega_n} = 0.1 \) or \( \dot{x}_0 = 0.1 \omega_n = 4 \text{ m/s} \)

---

\( 2.73 \)  

**Data:**  
\( D = 0.5625'' \),  
\( G = 11.5 \times 10^6 \text{ psi} \),  
\( f = 0.282 \text{ lb/in}^3 \)

\( f = 193 \text{ Hz} \),  
\( k = \text{26.4 lb/in} \)

\( k = \text{spring rate} = \frac{d^4 G}{8 D^3 N} \Rightarrow \frac{d^4 (11.5 \times 10^6)}{8 (0.5625^3)} = 26.4 \)

\( \text{or} \quad \frac{d^4}{N} = \frac{26.4 (8) (0.5625^3)}{11.5 \times 10^6} = 3.2686 \times 10^{-6} \quad (E.1) \)

\[
f = \frac{1}{2} \sqrt{\frac{k g}{\omega_n^2}}
\]

where  
\[
W = \left( \frac{\pi d^2}{4} \right) \pi D N \Rightarrow \frac{\pi^2}{4} \left( 0.5625 \right) \left( 0.282 \right) N d^2 = 0.391393 N d^2
\]

Hence  
\[
f = \frac{1}{2} \sqrt{\frac{26.4 (386.4)}{0.391393 N d^2}} = 193
\]

\( \text{or} \quad N d^2 = 0.174925 \quad (E.2) \)

Eqs. (E.1) and (E.2) yield  
\[
N = \frac{d^4}{3.2686 \times 10^{-6}} = \frac{0.174925}{d^2}
\]

\( \text{or} \quad d^6 = 0.571764 \times 10^{-6} \)

\( \text{or} \quad d = 0.911037 \times 10^{-1} = 0.0911037 \text{ inch} \)

Hence  
\[
N = \frac{0.174925}{d^2} = 21.075641
\]
2.74 Data: \( d = 0.5625'' \), \( G = 4 \times 10^6 \text{ psi} \), \( \rho = 0.1 \text{ lb/ft}^3 \)
\( f = 193 \text{ Hz} \), \( k = 26.4 \text{ lb/in} \)

\[ k = \text{spring rate} = \frac{d^4 G}{8 D^3 N} \Rightarrow \frac{d^4 (4 \times 10^6)}{8 (0.5625^3) N} = 26.4 \]

or \[ \frac{d^4}{N} = \frac{26.4 (8) (0.5625^3)}{4 \times 10^6} = 9.397266 \times 10^{-6} \quad (E.1) \]

\( f = \text{frequency} = \frac{1}{2} \sqrt{\frac{k}{W}} \)

where \[ W = \left( \frac{\pi d^2}{4} \right) \pi D N \rho = \frac{\pi^2}{4} (0.5625) (0.1) N d^2 \]
\[ = 0.138792 N d^2 \]

Hence \[ f = \frac{1}{2} \sqrt{\frac{26.4 (386.4)}{0.138792 N d^2}} = 193 \]

or \[ N d^2 = 0.493290 \quad (E.2) \]

Eqs. (E.1) and (E.2) yield

\[ N = \frac{d^4}{9.397266 \times 10^{-6}} = \frac{0.493290}{d^2} \]

or \[ d^6 = 4.635575 \times 10^{-6} \]

or \[ d = 0.129127 \text{ inch} \]

Hence \[ N = \frac{0.493290}{d^2} = 29.584728 \]

2.75 By neglecting the effect of self weight of the beam, and using a single degree of freedom model, the natural frequency of the system can be expressed as \[ \omega_n = \sqrt{\frac{k}{m}} \]

2-61
where \( m \) = mass of the machine, and
\[ k = \frac{3EI}{l^3} \]
where \( l \) = length, \( E \) = Young’s modulus, and \( I \) = area moment of inertia of the beam section.
Assuming \( E = 30 \times 10^6 \) psi for steel and \( 10.5 \times 10^6 \) psi for aluminum, we have
\[ (\omega_n)_{\text{steel}} = \left\{ \frac{3 \times (30 \times 10^6)}{m} \right\} \frac{I}{l^3} \]
\[ (\omega_n)_{\text{aluminum}} = \left\{ \frac{3 \times (10.5 \times 10^6)}{m} \right\} \frac{I}{l^3} \]
Ratio of natural frequencies:
\[ \frac{(\omega_n)_{\text{steel}}}{(\omega_n)_{\text{aluminum}}} = \left( \frac{30}{10.5} \right)^{1/2} = 1.6903 = \frac{1}{0.5916} \]
Thus the natural frequency is reduced to 59.16% of its value if aluminum is used instead of steel.
At equilibrium position,

\[ M = \text{mass of drum} = 500 \text{ kg} \]

\[ = (\pi r^2)(x)(1050) \]

\[ = \pi (0.5)^2 x (1050) \]

\[ \therefore x = \frac{500}{\pi (0.25)(1050)} = 0.6063 \text{ m} \]

Let the drum be displaced by a vertical distance \( x \) from its equilibrium position. Then the equation of motion can be expressed as:

\[ M \ddot{x} + \left( \text{reaction force due to the weight of salt water displaced due to } x \right) = 0 \]

or

\[ M \ddot{x} + (\pi r^2) x (1050 \times 9.81) = 0 \]

or

\[ 500 \ddot{x} + \pi (0.5)^2 x (1050 \times 9.81) = 0 \]

or

\[ \ddot{x} + \frac{0.25 \pi (1050 \times 9.81)}{500} \] \( x = 0 \)

or

\[ \ddot{x} + 16.18 \, x = 0 \]
from which the natural frequency of vibration can be determined as

\[ \omega_n = \sqrt{16.18} = 4.0224 \text{ rad/s} \]
From the equation of motion, we note

\[ m = 500 \text{ kg and spring force } F = \frac{1000 \times 3}{(0.025)^3} \text{ N} \]

(a) By equating the weight of the mass and the spring force,

\[ 500(9.81) = \frac{1000 \times 3}{(0.025)^3} x^3 \quad (1) \]

we find the static equilibrium position of the system as

\[ x_{st} = \frac{500(9.81)(0.025^3)}{1000} = 76.641 \times 10^{-6} \]

or

\[ x_{st} = 4.2477 \times 10^{-2} = 0.04248 \text{ m} \]

(b) The linearized spring constant, \( k \), about the static equilibrium position \( x_{st} \) is given by

\[ k = \left. \frac{dF}{dx} \right|_{x = x_{st}} = \frac{3000}{(0.025)^3} x^2 \bigg|_{x = x_{st}} \]

\[ = \frac{3000}{(0.025)^3} \left( 4.2477 \times 10^{-2} \right)^2 \]

\[ = \frac{3000 \cdot (4.2477)^2}{15.625 \times 10^{-6}} \cdot 10^{-4} \]

\[ = \frac{3 \times 10^3 \cdot (18.0429) \times 10^{-6}}{15.625 \times 10^{-6}} = 3.4642 \times 10^5 \text{ N/m} \]
(c) Natural frequency of vibration for small displacements:

\[ \omega_n = \sqrt{\frac{k}{m}} = \left( \frac{3.4642 \times 10^5}{500} \right)^{\frac{1}{2}} = 26.3218 \text{ rad/s} \]

(d) Natural frequency of vibration for small displacements when \( m = 600 \text{ kg} \):

In this case, the static equilibrium position is given by

\[ \bar{x}_{st} = \frac{600 (9.81) (0.025^3)}{1000} = 5.886 \times (0.025)^3 \]

\[ \bar{x}_{st} = 1.8055 \times 0.025 = 0.04514 \text{ m} \]

The linearized spring constant, \( k' \), about the static equilibrium position \( (\bar{x}_{st}) \) is given by

\[ k' = \frac{dF}{dx} \bigg|_{x=\bar{x}_{st}} = \frac{3000}{(0.025)^3} \left( \frac{\bar{x}_{st}}{0.025} \right)^2 \]

\[ = \frac{3000}{(0.025)^3} \left( 4.514 \times 10^{-2} \right)^2 \]

\[ = \frac{3000 \left( 20.3748 \times 10^{-4} \right)}{15.625 \times 10^{-6}} = 3.9120 \times 10^5 \text{ N/m} \]

Hence the natural frequency of vibration for small displacements:

\[ \omega_n = \sqrt{\frac{k'}{m}} = \left( \frac{3.9120 \times 10^5}{600} \right)^{\frac{1}{2}} = 25.3429 \text{ rad/s} \]
\( \text{acceleration} = a = -10 \, \text{m/s}^2 = \ddot{x} = \frac{d^2x}{dt^2} \) (1)

Integration of Eq. (1) w.r.t. time gives

\[ \dot{x} = \frac{dx}{dt} = -10 \, t + c_1 \] (2)

At the brakes are applied, \( t = 0 \) and \( \dot{x} = u = 100 \, \text{km/hour} \)

\[ u = \dot{x} (t=0) = \frac{100 \times 10^3}{60 \times 60} \quad \frac{m}{s} = 27.7778 \quad \frac{m}{s} = c_1 \]

\[ : \frac{dx}{dt} (t) = -10 \, t + 27.7778 \]

\( \frac{dx}{dt} = 0 \) when the vehicle stops and hence the time taken before the vehicle stops, \( t_0 \), is given by

\[ 0 = -10 \, t_0 + 27.7778 \]

or \( t_0 = 2.7778 \, \text{s} \)

The distance traveled before it stops is given by

\[ s = ut_0 + \frac{1}{2} at_0^2 \]

\[ = 27.7778 \times 2.7778 + \frac{1}{2} (-10) (2.7778)^2 \]

\[ = 77.1612 - 38.5808 \]

\[ = 38.5803 \, \text{m} \]
For hollow circular post,

\[ I_{xx} = I_{yy} = \frac{\pi}{4} (r_0^4 - r_1^4) \]

\[ = \frac{\pi}{4} (0.05^4 - 0.045^4) \]

\[ = 1.6878 \times 10^{-6} \text{ m}^4 \]

Effective length of post (for bending stiffness) is

\[ l_e = 2.0 - 0.2 = 1.8 \text{ m} \]

Bending stiffness of the post in xz-plane:

\[ k_{xz} = \frac{3EI_{yy}}{l_e^3} = \frac{3(207 \times 10^9)(1.6878 \times 10^{-6})}{(1.8)^3} \]

\[ = 179,719.4 \times 10^3 \text{ N/m} \]

Mass of the post = \[ m = \pi (r_0^2 - r_1^2) l \]

\[ = m = \pi (0.05^2 - 0.045^2)(2)(\frac{76500}{9.81}) = 23,273.8 \text{ kg} \]

Mass of traffic sign = \[ M = b d t \]

\[ = M = 0.75(0.4)(0.005)(\frac{76500}{9.81}) = 11,697.2 \text{ Kg} \]

Equivalent mass of a cantilever beam of mass \( m \) with an end mass \( M \) (from back of front cover):

\[ m_{eq} = M + 0.23 m = 11,697.2 + 0.23 (23,273.8) \]

\[ = 17,050.2 \text{ kg} \]

Natural frequency for vibration in xz plane:
\[ \omega_n = \left( \frac{k x_3}{m e g} \right)^{\frac{1}{2}} = \left( \frac{179.7194 \times 10^3}{17.0502} \right)^{\frac{1}{2}} \]

\[ = 102.6674 \text{ rad/s} \]

**Bending stiffness of the post in y3-plane:**

\[ k_{y3} = \frac{3 E I_{y3}}{l_c^3} = \frac{3 (207 \times 10^9) (1.6878 \times 10^{-6})}{(1.8)^3} \]

\[ = 179.7194 \times 10^3 \text{ N/m} \]

**Natural frequency for vibration in y3-plane:**

\[ \omega_n = \left( \frac{k_{y3}}{m e g} \right)^{\frac{1}{2}} = \left( \frac{179.7194 \times 10^3}{17.0502} \right)^{\frac{1}{2}} \]

\[ = 102.6674 \text{ rad/s} \]
For hollow circular post,

\[ I_{xx} = I_{yy} = \frac{\pi}{4} \left( r_0^4 - r_1^4 \right) \]
\[ = \frac{\pi}{4} \left( 0.05^4 - 0.045^4 \right) \]
\[ = 1.6878 \times 10^{-6} \text{ m}^4 \]

Effective length of post (for bending stiffness) is
\[ l_e = 2.0 - 0.2 = 1.8 \text{ m} \]

Bending stiffness of the post in \( xz \)-plane:
\[ k_{xz} = \frac{3EI_{yy}}{l_e^3} = \frac{3 \left( 111 \times 10^9 \right) \left( 1.6878 \times 10^{-6} \right)}{1.8^3} \]
\[ = 96.3727 \times 10^3 \text{ N/m} \]

Mass of the post = \( m = \pi \left( r_0^2 - r_1^2 \right) l_s \)
\[ = \pi \left( 0.05^2 - 0.045^2 \right) (2) \left( \frac{80100}{9.81} \right) = 24.3690 \text{ kg} \]

Mass of traffic sign = \( M = b d t g \)
\[ = M = 0.75 (0.4) (0.005) \left( \frac{80100}{9.81} \right) = 12.2476 \text{ kg} \]

Equivalent mass of a cantilever beam of mass \( m \) with an end mass \( M \) (from back of front cover):
\[ m_{eq} = M + 0.23 m = 12.2476 + 0.23 (24.3690) \]
\[ = 17.8525 \text{ kg} \]

Natural frequency for vibration in \( xz \) plane:

2-70
\[ \omega_n = \left( \frac{k \times g}{m_{eq}} \right)^{\frac{1}{2}} = \left( \frac{96,372 \times 10^3}{17,8525} \right)^{\frac{1}{2}} = 73.4729 \text{ rad/s} \]

Bending stiffness of the post in y-z plane:

\[ K_{yz} = \frac{3EI_{xx}}{L_e^3} = \frac{3(111 \times 10^9)(1.6878 \times 10^{-6})}{(1.8)^3} = 96,3727 \times 10^3 \text{ N/m} \]

Natural frequency for vibration in y-z plane:

\[ \omega_n = \left( \frac{K_{yz}}{m_{eq}} \right)^{\frac{1}{2}} = \left( \frac{96,3727 \times 10^3}{17,8525} \right)^{\frac{1}{2}} = 73.4729 \text{ rad/s} \]
Any applied moment \( M_t \) at the disk will be felt by every point along the stepped shaft. As such, the two steps of diameters \( d_1 \) and \( d_2 \) (with lengths \( l_1 \) and \( l_2 \)) act as series torsional springs. Torsional spring constants of steps 1 and 2 are given by

\[
(1) \quad k_{t1} = \frac{G \cdot I_{o1}}{l_1} \quad ; \quad I_{o1} = \text{polar moment of inertia of shaft 1} = \frac{\pi d_1^4}{32}
\]

\[
(2) \quad k_{t2} = \frac{G \cdot I_{o2}}{l_2} \quad ; \quad I_{o2} = \text{polar moment of inertia of shaft 2} = \frac{\pi d_2^4}{32}
\]

Equivalent torsional spring constant, \( k_{teq} \), is given by

\[
\frac{1}{k_{teq}} = \frac{1}{k_{t1}} + \frac{1}{k_{t2}}
\]

or

\[
k_{teq} = \frac{k_{t1} \cdot k_{t2}}{k_{t1} + k_{t2}}
\]
Natural frequency of heavy disk, of mass moment of inertia $J$, can be found as

$$\omega_n = \sqrt{\frac{k_t e^2}{J}} = \sqrt{\frac{k_{t1} k_{t2}}{J (k_{t1} + k_{t2})}}$$

where $k_{t1}$ and $k_{t2}$ are given by Eqs. (1) and (2).
(a) Equation of motion of simple pendulum for small angular motions is given by

\[
\ddot{\theta} + \frac{g_{\text{mars}}}{l} \theta = 0
\]

and hence the natural frequency of vibration is

\[
\omega_n = \sqrt{\frac{g_{\text{mars}}}{l}} = \sqrt{\frac{0.376 \times 9.81}{1}} = 1.9206 \text{ rad/s}
\]

(b) Solution of Eq. (1) can be expressed, similar to Eq. (2.23), as

\[
\theta(t) = A_0 \sin (\omega_n t + \phi_0)
\]

with

\[
A_0 = \sqrt{\theta_0^2 + (\frac{\dot{\theta}_0}{\omega_n})^2} = \sqrt{(0.08727)^2 + 0^2} = 0.08727 \text{ rad}
\]

since \( \theta_0 = 5^\circ = 0.08727 \text{ rad} \) and \( \dot{\theta}_0 = 0 \).

and \( \phi_0 = \tan^{-1} \left( \frac{\theta_0}{\omega_n} \right) = \tan^{-1} \left( \frac{0.08727 \times 1.9206}{0} \right) \)

\( = \tan^{-1} (\infty) = 90^\circ \) or \( 1.5708 \text{ rad} \)

\( \therefore \theta(t) = 0.08727 \sin (1.9206 t + 1.5708) \text{ rad} \)

\( \dot{\theta}(t) = 0.08727 (1.9206) \cos (1.9206 t + 1.5708) \)

\( = 0.1676 \cos (1.9206 t + 1.5708) \text{ rad/s} \)

Maximum velocity = \( \dot{\theta}_{\text{max}} = 0.1676 \text{ rad/s} \)

(c) \( \ddot{\theta}(t) = -0.1676 (1.9206) \sin (1.9206 t + 1.5708) \)

\( = -0.3219 \sin (1.9206 t + 1.5708) \text{ rad/s}^2 \)

Maximum acceleration = \( \ddot{\theta}_{\text{max}} = 0.3219 \text{ rad/s}^2 \)
(a) Equation of motion of simple pendulum for small angular motions is

\[ \ddot{\theta} + \frac{g_{\text{moon}}}{\ell} \theta = 0 \]  

(1)

Natural frequency of vibration is

\[ \omega_n = \sqrt{\frac{g_{\text{moon}}}{\ell}} = \sqrt{\frac{1.6263}{1}} = 1.2753 \text{ rad/s} \]

(b) Solution of Eq. (1) can be written as (similar to Eq. (2.23)):

\[ \theta(t) = A_0 \sin(\omega_n t + \phi_0) \]  

(2)

where

\[ A_0 = \left\{ \theta_0 + \left( \frac{\dot{\theta}_0}{\omega_n} \right) \right\}^{\frac{1}{2}} = \left\{ 0.08727 + 0 \right\}^{\frac{1}{2}} \]

= 0.08727 rad

and

\[ \phi_0 = \tan^{-1}\left( \frac{\theta_0}{\dot{\theta}_0} \right) = \tan^{-1}(0) = 90^\circ \text{ or } 1.5708 \text{ rad} \]

\[ \therefore \theta(t) = 0.08727 \sin(1.2753 t + 1.5708) \text{ rad} \]

\[ \dot{\theta}(t) = 0.08727(1.2753) \cos(1.2753 t + 1.5708) \]

= 0.1113 \cos(1.2753 t + 1.5708) \text{ rad/s} \]

\[ \dot{\theta}_{\text{max}} = 0.1113 \text{ rad/s} \]

(c) \[ \ddot{\theta}(t) = -0.1113(1.2753) \sin(1.2753 t + 1.5708) \]

= -0.1419 \sin(1.2753 t + 1.5708) \text{ rad/s}^2 \]

\[ \therefore \ddot{\theta}_{\text{max}} = 0.1419 \text{ rad/s}^2 \]
For free vibration, apply Newton's second law of motion:

\[ m l \ddot{\theta} + mg \sin \theta = 0 \]  \hspace{1cm} (E.1)

For small angular displacements, Eq. (E.1) reduces to

\[ m l \ddot{\theta} + mg \theta = 0 \]  \hspace{1cm} (E.2)

or \[ \ddot{\theta} + \omega_n^2 \theta = 0 \]  \hspace{1cm} (E.3)

where \[ \omega_n = \sqrt{\frac{g}{l}} \]  \hspace{1cm} (E.4)

Solution of Eq. (E.3) is:

\[ \theta(t) = \theta_o \cos \omega_n t + \frac{\dot{\theta}_o}{\omega_n} \sin \omega_n t \]  \hspace{1cm} (E.5)

where \( \theta_o \) and \( \dot{\theta}_o \) denote the angular displacement and angular velocity at \( t=0 \). The amplitude of motion is given by

\[ \Phi = \left( \theta_o^2 + \left( \frac{\dot{\theta}_o}{\omega_n} \right)^2 \right)^{1/2} \]  \hspace{1cm} (E.6)

Using \( \Phi = 0.5 \) rad, \( \theta_o = 0 \) and \( \dot{\theta}_o = 1 \) rad/s, Eq. (E.6) gives

\[ 0.5 = \frac{\dot{\theta}_o}{\omega_n} = \frac{1}{\omega_n} \] or \( \omega_n = 2 \) rad/s
The system of Fig. (A) can be drawn in equivalent form as shown in Fig. (B) where both pulleys have the same radius \( r_1 \). We notice in Fig. (B) that vibration can take place in only one way with one pulley moving clockwise and the other moving counterclockwise.

When pulleys rotate in opposite directions, \( \theta_1 = \frac{J_2}{J_1} \).

The spring force, which has the same value on either pulley is \(-k_1(\theta_1 + \theta_2)\) where \( k_1 \) = torsional spring constant of the system. Equation of motion is

\[
J_1 \ddot{\theta}_1 + k_1 (\theta_1 + \theta_2) = 0 \quad \text{and} \quad J_2 \ddot{\theta}_2 + k_1 (\theta_1 + \theta_2) = 0
\]

i.e., \( J_1 \ddot{\theta}_1 + \frac{J_1}{J_2} \theta_1 = 0 \) \quad \text{and} \quad \frac{J_1}{J_2} \ddot{\theta}_2 + \theta_2 = 0

Either of these equations gives

\[
\omega = \sqrt{\frac{k_1 (\frac{J_1}{J_2} + \frac{J_2}{J_1})}{J_1}} \quad \text{--- (E1)}
\]

Here \( J_1 = 0.25 \) = 0.05 kg·m²,

\( J_2 = \frac{J_2}{J_1} (= \text{speed ratio})^2 = 0.2 (\frac{1}{4})^2 = 0.0125 \text{ kg·m}^2 \)

\[ k = 454.7935 \text{ N/m}. \]

\[ k = \frac{\Delta M}{\Delta \theta} = \frac{\text{(force in springs)}}{\text{due to } \Delta \theta} = \frac{r_1}{r_2} = 2 \frac{r_1}{r_2} \]

\[ = 2 \times \frac{(125 \text{ kg·m/s}^2)}{1000} = 0.025 \text{ N·m/rad} \]

\[ \omega = 12 \pi \text{ rad}, \quad \text{gives, for } \omega = 12 \pi \text{ rad}, \quad k = 454.7935 \text{ N/m}. \]

\*The other possible motion is rotation of the two pulleys as a whole (as rigid body) in same direction. This will have a natural frequency of zero. See section 5.7.
\[ m \ddot{\theta} + mg \sin \theta = 0 \]

For small \( \theta \), \( m \ddot{\theta} + mg \theta = 0 \)

\[ \omega_n = \frac{2 \pi}{T} = \frac{2 \pi}{\sqrt{\frac{9.81}{0.5}}} = 1.4185 \text{ sec} \]

\[ \tau_n = \frac{2 \pi}{\omega_n} = \frac{2 \pi}{\sqrt{\frac{9.81}{0.5}}} = 1.4185 \text{ sec} \]

\[ \begin{align*}
(a) & \quad \omega_n = \frac{\sqrt{g}}{\sqrt{l}} \\
(b) & \quad m l^2 \ddot{\theta} + k a^2 \sin \theta + mg l \sin \theta = 0 ; \quad m l^2 \ddot{\theta} + (ka^2 + mg l) \theta = 0 \\
(c) & \quad m l^2 \ddot{\theta} + k a^2 \sin \theta - mg l \sin \theta = 0 ; \quad m l^2 \ddot{\theta} + (ka^2 - mg l) \theta = 0
\end{align*} \]

Configuration (b) has the highest natural frequency.

\[ \begin{align*}
m &= \text{mass of a panel} = (5 \times 12) (3 \times 12) (1) \left( \frac{0.283}{386.4} \right) = 1.5820 \\
J_0 &= \text{mass moment of inertia of panel about } x-\text{axis} = \frac{m}{12} (a^2 + b^2) \\
&= \frac{1.5820}{12} (1^2 + 36^2) = 170.9878 \\
I_0 &= \text{polar moment of inertia of rod} = \frac{\pi}{32} d^4 = \frac{\pi}{32} (1)^4 = 0.008175 \text{ in}^4
\end{align*} \]
\[ k_t = \frac{G I_0}{\ell} = \frac{(3.8 \times 10^8) \times (0.098175)}{12} = 3.1089 \times 10^4 \text{ lb-in/rad} \]

\[ \omega_n = \left( \frac{k_t}{J_0} \right)^{\frac{1}{2}} = \left( \frac{3.1089 \times 10^4}{170.9878} \right)^{\frac{1}{2}} = 13.4841 \text{ rad/sec} \]

2.89

\( I_0 = \text{polar moment of inertia of cross section of shaft AB} \)

\[ = \frac{\pi}{32} d^4 = \frac{\pi}{32} (1)^4 = 0.098175 \text{ in}^4 \]

\[ k_t = \text{torsional stiffness of shaft AB} = \frac{G I_0}{\ell} \]

\[ = \left( \frac{12 (10^6)}{6} \right) \times (0.098175) = 19.635 \times 10^4 \text{ lb-in/rad} \]

\( J_0 = \text{mass moment of inertia of the three blades about y-axis} \)

\[ = 3 J_0 |_{PQ} = 3 \left( \frac{1}{3} m \ell^2 \right) = m \ell^2 = \left( \frac{2}{386.4} \right) (12)^2 = 0.7453 \]

Torsional natural frequency:

\[ \omega_n = \left( \frac{k_t}{J_0} \right)^{\frac{1}{2}} = \left( \frac{19.635 \times 10^4}{0.7453} \right)^{\frac{1}{2}} = 513.2747 \text{ rad/sec} \]

2.90

\( J_0 = \text{mass moment of inertia of the ring} = 1.0 \text{ kg-m}^2 \)

\( I_{os} = \text{polar moment of inertia of the cross section of steel shaft} \)

\[ = \frac{\pi}{32} (d_{os}^4 - d_{bs}^4) = \frac{\pi}{4} (0.05^4 - 0.04^4) = 36.2266 \times 10^{-8} \text{ m}^4 \]

\( I_{ob} = \text{polar moment of inertia of cross section of brass shaft} \)

\[ = \frac{\pi}{32} (d_{ob}^4 - d_{bb}^4) = \frac{\pi}{32} (0.04^4 - 0.03^4) = 17.1806 \times 10^{-8} \text{ m}^4 \]

\( k_{ts} = \text{torsional stiffness of steel shaft} \)

\[ = \frac{G_s I_{os}}{\ell} = \frac{(80 (10^9)) \times (36.2266 \times 10^{-8})}{2} = 14490.64 \text{ N-m/rad} \]

\( k_{tb} = \text{torsional stiffness of brass shaft} \)

\[ = \frac{G_b I_{ob}}{\ell} = \frac{(40 (10^9)) \times (17.1806 \times 10^{-8})}{2} = 3436.12 \text{ N-m/rad} \]

\( k_{teq} = k_{ts} + k_{tb} = 17,926.76 \text{ N-m/rad} \)

Torsional natural frequency:

\[ \omega_n = \sqrt{\frac{k_{teq}}{J_0}} = \sqrt{\frac{17926.76}{1}} = 133.8908 \text{ rad/sec} \]

Natural time period:

\[ \tau_n = \frac{2 \pi}{\omega_n} = \frac{2 \pi}{133.8908} = 0.04693 \text{ sec} \]
2.91

Kinetic energy of system is

\[ T = T_{rod} + T_{bob} = \frac{1}{2} \left( \frac{1}{3} m l^2 \right) \dot{\theta}^2 + \frac{1}{2} M l^2 \dot{\theta}^2 \]

Potential energy of system is

(since mass of the rod acts through its center)

\[ U = U_{rod} + U_{bob} = \frac{1}{2} m g l (1 - \cos \theta) + \frac{1}{2} M g l (1 - \cos \theta) \]

Equation of motion:

\[ \frac{d}{dt} (T + U) = 0 \]

i.e.

\[ (M + \frac{m}{3}) l^2 \ddot{\theta} + (M + \frac{m}{3}) g l \sin \theta = 0 \]

For small angles,

\[ \ddot{\theta} + \frac{(M + \frac{m}{3}) g}{(M + \frac{m}{3}) l} \theta = 0 \]

\[ \omega_n = \sqrt{\frac{(M + \frac{m}{3}) g}{(M + \frac{m}{3}) l}} \]

2.92

For the shaft,

\[ J = \frac{\pi d^4}{32} = \frac{\pi (0.05)^4}{32} = 61.3594 \times 10^{-8} \ m^4 \]

\[ k_t = \frac{G J}{l} = \frac{(0.793 \times 10^{11}) (61.3594 \times 10^{-8})}{32} = 24329.002 \ N \cdot m/\text{rad} \]

For the disc,

\[ J_\theta = \frac{MD^2}{8} = \left( \frac{3}{4} \pi D^2 h \right) \frac{D^2}{8} = \frac{3}{32} \pi D^4 h \]

\[ = \frac{(7.83 \times 10^3) \pi (1)^4 (0.1)}{32} = 76.871 \ \text{kg} \cdot \text{m}^2 \]

\[ \omega_n = \sqrt{\frac{k_t}{J_\theta}} = \left( \frac{24329.002}{76.871} \right)^{1/2} = 17.7902 \ \text{rad/sec} \]

2.93

Equation of motion

\[ \ddot{\theta} = -W d \theta - 2 \kappa \left( \frac{2 l}{3} \theta \right) \frac{2 l}{3} - k_t \theta \]

Where

\[ J_A = J_G + m d^2 = \frac{1}{12} m l^2 + m \frac{l^2}{36} \]

\[ = \frac{1}{9} m l^2 \]

\[ \therefore \ \frac{m l^2}{9} \ddot{\theta} + (m g d + 2 \kappa \frac{2 l}{9} \frac{l^2}{9} + \frac{8 \kappa l^2}{9} + k_t) \theta = 0 \]

\[ \omega_n = \sqrt{\frac{(m g d + 2 \frac{2 l}{9} \kappa l^2 + \frac{8 \kappa l^2}{9} l^2 + k_t) g}{m l^2}} = \sqrt{\frac{9 m g d + 10 \kappa l^2 + 9 \kappa l^2}{m l^2}} \]

2-80
For given data,
\[ \omega_n = \sqrt{\frac{9(10)(9.81)(5/6) + 10(2000)(5)^2 + 9(1000)}{10(5)^2}} = 45.15 \text{ rad/sec} \]

Let angular displacement = \( \theta \)

Equation of motion:
\[ J_c \ddot{\theta} + k_1(R+a)^2 \theta + k_2(R+a)^2 \theta = 0 \]
\[ \omega_n = \sqrt{\frac{(k_1 + k_2)(R+a)^2}{J_c}} = \sqrt{\frac{(k_1 + k_2)(R+a)^2}{1.5 m R^2}} \text{ (E1)} \]

Equation (E1) shows that \( \omega_n \) increases with the value of \( a \).

\[ \therefore \omega_n \text{ will be maximum when } a = R. \]

Net \( g \) acting on the pendulum = \( 9.81 - 5 = 4.81 \text{ m/sec}^2 = g_n \)
\[ \omega_n = \sqrt{\frac{g_n}{l}} = \sqrt{\frac{4.81}{5}} = 3.1016 \text{ rad/sec} \]
\[ \tau_n = \frac{2\pi}{\omega_n} = \frac{2.0258}{\text{sec}} \]

Equation of motion:
\[ J_0 \ddot{\theta} = -k_t \theta - (k_1 a^2 \theta - k_2 l \theta) \]
\[ \text{where } J_0 = \frac{1}{2} ml^2 + m \left( \frac{l}{2} \right)^2 = \frac{1}{2} ml^2 \]
\[ \therefore \frac{1}{3} ml^2 \ddot{\theta} + (k_t + k_1 a^2 + k_2 l^2) \theta = 0 \]
\[ \omega_n = \sqrt{\frac{3(k_t + k_1 a^2 + k_2 l^2)}{ml^2}}^{1/2} \]

\[ J_0 = J_G + mb^2 = \frac{1}{2} ma^2 + mb^2 \]

Equation of motion:
\[ J_0 \ddot{\theta} + mg b \theta = 0 \]
\[ \omega_n = \sqrt{\frac{mg b}{J_0}} = \sqrt{\frac{2gb}{a^2 + 2b^2}} \]
\[ \omega_n \frac{d\omega_n}{db} = \frac{1}{2} \left( \frac{2gb}{a^2 + 2b^2} \right)^{-1/2} \left\{ \frac{(a^2 + 2b^2)(2gb - 2gb)(4b)}{(a^2 + 2b^2)^2} \right\} = 0 \]
\[ i.e., \quad b = \pm \frac{a}{\sqrt{2}} \]

\[ \omega_n \bigg|_b = + \frac{a}{\sqrt{2}} = \sqrt{\frac{2g}{\sqrt{2}} \frac{a}{\sqrt{2}}} = \sqrt{\frac{g}{\sqrt{2}}} a \]

\[ b = - \frac{a}{\sqrt{2}} \text{ gives imaginary value for } \omega_n. \]

Since \( \omega_n = 0 \) when \( b = 0 \), we have \( \omega_n \big|_{\text{max}} \) at \( b = \frac{a}{\sqrt{2}} \).

Let \( \theta \) be measured from static equilibrium position so that gravity force need not be considered.

(a) Newton’s second law of motion:

\[ J_0 \ddot{\theta} = -3k \left( \theta \frac{L}{4} \right) \frac{L}{4} - k \left( \theta \frac{3L}{4} \right) \left( \frac{3L}{4} \right) \text{ or } J_0 \ddot{\theta} + \frac{3}{4} k \ell^2 \theta = 0 \]

(b) D’Alembert’s principle:

\[ M(t) - J_0 \ddot{\theta} = 0 \text{ or } -3k \left( \theta \frac{L}{4} \right) \left( \frac{L}{4} \right) - k \left( \theta \frac{3L}{4} \right) \left( \frac{3L}{4} \right) - J_0 \ddot{\theta} = 0 \]

\[ \text{or } J_0 \ddot{\theta} + \frac{3}{4} k \ell^2 \theta = 0 \]

(c) Principle of virtual work:

Virtual work done by spring force:

\[ \delta W_s = -3k \left( \theta \frac{L}{4} \right) \left( \frac{L}{4} \delta \theta \right) - k \left( \theta \frac{3L}{4} \right) \left( \frac{3L}{4} \delta \theta \right) \]

Virtual work done by inertia moment = \(- (J_0 \ddot{\theta}) \delta \theta \)

Setting total virtual work done by all forces/moments equal to zero, we obtain

\[ J_0 \ddot{\theta} + \frac{3}{4} k \ell^2 \theta = 0 \]
Torsional stiffness of the post (about $z$-axis):

$$k_e = \frac{\pi G}{2 l_e} \left( r_0^4 - r_i^4 \right) = \frac{\pi \left( 69.3 \times 10^9 \right) \left( 0.054 - 0.045^4 \right)}{2 \left( 1.8 \right)} = 148.7161 \times 10^3 \text{ N-m}$$

Mass moment of inertia of the sign about the $z$-axis:

$$J_{\text{sign}} = \frac{M}{12} \left( d^2 + b^2 \right)$$

with

Mass of traffic sign,

$$M = b d t f = 0.75 \times 0.4 \times 0.005 \times \frac{76500}{9.81} = 11.6972 \text{ Kg}$$

Hence

$$J_{\text{sign}} = \frac{11.6972 \left( 0.4^2 + 0.75^2 \right)}{12} = 0.7043 \text{ Kg-m}^2$$

Mass moment of inertia of the post about the $z$-axis:

$$J_{\text{post}} = \frac{m}{8} \left( d_0^2 + d_i^2 \right)$$

with

$$d_0 = 2 r_0 = 0.1 \text{ m}, \quad d_i = 2 r_i = 2 \times 0.45 = 0.09 \text{ m}$$

and

Mass of the post,

$$m = \pi \left( r_0^2 - r_i^2 \right) l f = \frac{\pi \left( 0.05^2 - 0.045^2 \right) \left( 2 \right) \left( 76500 \right) \left( 9.81 \right)}{2} = 23.2738 \text{ Kg}$$

2-83
Hence
\[ J_{\text{post}} = \frac{23.2738 \left(0.10^2 + 0.09^2\right)}{8} = 0.052657 \text{ kg} \cdot \text{m}^2 \]

Equivalent mass moment of inertia of the post (\( J_{\text{eff}} \)) about the location of the sign:
\[ J_{\text{eff}} = \frac{J_{\text{post}}}{3} = \frac{0.052657}{3} = 0.017552 \text{ kg} \cdot \text{m}^2 \]

(Derivation given below)

Natural frequency of torsional vibration of the traffic sign about the \( z \)-axis:
\[ \omega_n = \left(\frac{k_t}{J_{\text{sign}} + J_{\text{eff}}}\right)^{\frac{1}{2}} \]
\[ = \left(\frac{148.7161 \times 10^3}{0.7043 + 0.017552}\right)^{\frac{1}{2}} \]
\[ = 453.89 \text{ rad/s} \]

**Derivation:**

Effect of the mass moment of inertia of the post or shaft (\( J_{\text{eff}} \)) on the natural frequency of vibration of a shaft carrying end mass moment of inertia (\( J_{\text{sign}} \)):

Let \( \dot{\theta} \) be the angular velocity of the end mass moment of inertia (\( J_{\text{sign}} \)) during vibration. Assume a linear variation of the angular velocity of the shaft (post) so that at a distance \( x \) from the fixed end, the angular
velocity is given by \( \dot{\theta} x \).

The total kinetic energy of the shaft (post) is given by:

\[
\mathcal{T}_{\text{post}} = \frac{1}{2} \int_0^l \left( \frac{\dot{\theta} x}{l} \right) \left( \frac{j_{\text{post}}}{l} \right) \, dx
\]

\[
= \frac{1}{2} \frac{j_{\text{post}}}{3} \left( \frac{\dot{\theta}}{l} \right)^2
\]

This shows that the effective mass moment of inertia of the shaft (post) at the end is \( \frac{j_{\text{post}}}{3} \).
Torsional stiffness of the post (about \( z \)-axis):

\[
k_t = \frac{\pi G}{2 \ell_e} (r_o^4 - r_i^4)
\]

\[
= \frac{\pi (41.4 \times 10^9)(0.05^4 - 0.045^4)}{2 (1.8)}
\]

\[
= 77.6399 \times 10^3 \text{ N-m}
\]

Mass moment of inertia of the sign about the \( z \)-axis:

\[
J_{\text{sign}} = \frac{M}{12} (d^2 + b^2)
\]

with

mass of traffic sign = \( M = b d t g \)

\[
= 0.75 (0.4) (0.005) \left( \frac{80}{9.81} \right) = 12.2476 \text{ Kg}
\]

Hence

\[
J_{\text{sign}} = \frac{12.2476 (0.4^2 + 0.75^2)}{12} = 0.7374 \text{ Kg-m}^2
\]

Mass moment of inertia of the post about the \( z \)-axis:

\[
J_{\text{post}} = \frac{m}{8} (d_o^2 + d_i^2)
\]

with \( d_o = 2r_o = 0.10 \text{ m} \), \( d_i = 2r_i = 2(0.045) = 0.09 \text{ m} \)

Mass of the post = \( m = \pi (r_o^2 - r_i^2) \ell f \)

\[
= \pi (0.05^2 - 0.045^2) (2) \left( \frac{76500}{9.81} \right) = 24.3690 \text{ Kg}
\]

\[2.100\]
Hence
\[ J_{\text{post}} = \frac{24.3690}{8} \left( 0.10^2 + 0.09^2 \right) = 0.055135 \text{ kg} \cdot \text{m}^2 \]

Equivalent mass moment of inertia of the post (\( J_{\text{eff}} \)) about the location of the sign:
\[ J_{\text{eff}} = \frac{J_{\text{post}}}{3} = \frac{0.055135}{3} = 0.018378 \text{ kg} \cdot \text{m}^2 \]

(Derivation given in the solution of Problem 2.79)

Natural frequency of torsional vibration of the traffic sign about the \( z \)-axis:
\[ \omega_n = \left( \frac{k_t}{J_{\text{sign}} + J_{\text{eff}}} \right)^{\frac{1}{2}} \]
\[ = \left( \frac{77.6399 \times 10^3}{0.7374 + 0.018378} \right)^{\frac{1}{2}} \]
\[ = 320.5127 \text{ rad/s} \]
Assume the end mass $m_1$ to be a point mass. Then the mass moment of inertia of $m_1$ about the pivot point is given by

$$I_1 = m_1 l^2$$

For the uniform bar of length $l$ and mass $m_2$, its mass moment of inertia about the pivot $O$ is given by

$$I_2 = \frac{1}{12} m_2 l^2 + m_2 \left(\frac{l^2}{2}\right)^2 = \frac{1}{3} m_2 l^2$$

Inertial moment about pivot point $O$ is given by

$$I_0 \dot{\theta} + m_2 g \cdot \frac{l}{2} \sin \theta + m_1 g \cdot l \cos \theta = 0 \quad (3)$$

where

$$I_0 = I_1 + I_2 = m_1 l^2 + \frac{1}{3} m_2 l^2 \quad (4)$$

for small angular displacement, $\sin \theta \approx \theta$ and Eq. (3) can be expressed as

$$(m_1 l^2 + \frac{1}{3} m_2 l^2) \ddot{\theta} + (m_1 g l + \frac{m_2 g l}{2}) \theta = 0$$

or

$$\ddot{\theta} + \frac{3(2 m_1 g l + m_2 g l)}{2 (3 m_1 l^2 + m_2 l^2)} \theta = 0$$
or \[ \ddot{\theta} + \frac{g l}{l^2} \left( \frac{6m_1 + 3m_2}{6m_1 + 2m_2} \right) \theta = 0 \]

or \[ \ddot{\theta} + \frac{g}{l} \left( \frac{6m_1 + 3m_2}{6m_1 + 2m_2} \right) \theta = 0 \] (5)

By expressing Eq. (5) as \( \ddot{\theta} + \omega_n^2 \theta = 0 \), the natural frequency of vibration of the system can be expressed as

\[ \omega_n = \sqrt{\frac{g}{l} \left( \frac{6m_1 + 3m_2}{6m_1 + 2m_2} \right)} \] (6)
Equation of motion for the angular motion of the forearm about the pivot point 0:

\[ I_0 \ddot{\theta}_t + m_2 g b \cos \theta_t + m_1 g \frac{b}{2} \cos \theta_t \]

\[- F_2 a_2 + F_1 a_1 = 0 \quad (1)\]

where \( \theta_t \) is the total angular displacement of the forearm, \( I_0 \) is the mass moment of inertia of the forearm and the mass carried:

\[ I_0 = m_2 b^2 + \frac{1}{3} b^2 m_1 \quad (2)\]

and the forces in the biceps and triceps muscles (\( F_2 \) and \( F_1 \)) are given by

\[ F_2 = -c_2 \dot{\theta}_t \quad (3)\]

\[ F_1 = c_1 \ddot{x} = c_1 a_1 \dot{\theta}_t \quad (4)\]

where the linear velocity of the triceps can be expressed as

\[ \dot{x} \approx a_1 \dot{\theta}_t \quad (5)\]

Using Eqs. (2) - (4), Eq. (1) can be rewritten as

\[ I_0 \ddot{\theta}_t + (m_2 g b + \frac{1}{2} m_1 g b) \cos \theta_t \]

\[ + c_2 a_2 \dot{\theta}_t + c_1 a_1^2 \dot{\theta}_t = 0 \quad (6)\]

Let the forearm undergo small angular displacement (\( \theta \)) about the static equilibrium position, \( \bar{\theta} \), so that
\[
\theta_t = \bar{\theta} + \theta 
\]

Using Taylor's series expansion of \( \cos \theta_t \) about \( \bar{\theta} \), the static equilibrium position, can be expressed as (for small values of \( \theta \)):

\[
\cos \theta_t = \cos (\bar{\theta} + \theta) \approx \cos \bar{\theta} - \theta \sin \bar{\theta} 
\]

Using \( \ddot{\theta}_t = \ddot{\bar{\theta}} \) and \( \dot{\theta}_t = \dot{\bar{\theta}} \), Eq. (8) can be expressed as

\[
I_0 \ddot{\bar{\theta}} + (m_2 g b + \frac{1}{2} m_1 g b) (\cos \bar{\theta} - \sin \bar{\theta} \theta) + c_2 a_2 (\bar{\theta} + \theta) + c_1 a_1^2 \dot{\bar{\theta}} = 0 
\]

or

\[
I_0 \ddot{\bar{\theta}} + (m_2 g b + \frac{1}{2} m_1 g b) \cos \bar{\theta} - \sin \bar{\theta} (m_2 g b + \frac{1}{2} m_1 g b) \theta + c_2 a_2 \bar{\theta} + c_2 a_2 \theta + c_1 a_1^2 \dot{\bar{\theta}} = 0 
\]

Noting that the static equilibrium equation of the forearm at \( \theta_t = \bar{\theta} \) is given by

\[
(m_2 g b + \frac{1}{2} m_1 g b) \cos \bar{\theta} + c_2 a_2 \bar{\theta} = 0 
\]

In view of Eq. (10), Eq. (9) becomes

\[
(m_2 b^2 + \frac{1}{3} b^2 m_1) \ddot{\theta} + c_1 a_1^2 \dot{\theta} + \left\{ c_2 a_2 - \sin \bar{\theta} g b (m_2 + \frac{1}{2} m_1) \right\} \theta = 0 
\]

which denotes the equation of motion of the forearm.
The undamped natural frequency of the forearm can be expressed as

\[ \omega_n = \sqrt{\frac{c_2 a_2 - \sin \theta \ g b \ (m_2 + \frac{1}{2} m_1)}{b^2 \ (m_2 + \frac{1}{2} m_1)}} \]  

(12)
2.103

(a) \( 100 \ddot{v} + 20 \dot{v} = 0 \)

Using a solution similar to Eqs. (2.52) and (2.53), we find:

Free vibration response: \( v(t) = v(0) e^{-\frac{20}{100} t} \)

Time constant: \( \tau = \frac{100}{20} = 5 \text{ sec} \)

(b) \( v(t) = v_h(t) + v_p(t) \)

with \( v_h(t) = A e^{-\frac{20}{100} t} \) where \( A \) = constant

and \( v_p(t) = C = \text{constant} \)

Substitution in the Equation of motion gives

\( 100(0) + 20 C = 10 \) or \( C = \frac{1}{2} \)

\[ \begin{align*}
  v(t) & = A e^{-\frac{20}{100} t} + \frac{1}{2} \\
  v(0) & = A e^0 + \frac{1}{2} = 10 \quad \text{or} \quad A = \frac{19}{2}
\end{align*} \]

Total response:

\[ \begin{align*}
  v(t) & = \frac{19}{2} e^{-\frac{20}{100} t} + \frac{1}{2} \\
  \text{Free vibration response:} & = e^{-\frac{20}{100} t} \\
  \text{Homogeneous solution:} & = \frac{19}{2} e^{-\frac{20}{100} t} \\
  \text{Time constant:} & = \frac{100}{20} = 5 \text{ sec}
\end{align*} \]
(c) Free vibration response:

\[ v(t) = v(0) \cdot e^{\frac{20}{100} t} \]

This solution grows with time.

No time constant can be found.

(d) Free vibration solution:

\[ w(t) = 0.5 - \frac{50}{500} t = 0.5 e^{-0.1 t} \]

Time constant = \( \tau = \frac{50}{50} = 10 \).
Let \( t = 0 \) when force is released.

Before the force is released, the system is at rest so that

\[
F = kx; \quad t \leq 0
\]

\[
\alpha x(0) = \frac{F}{k} \quad \alpha 0.1 = \frac{5000}{k}
\]

\[
\therefore k = 5000 \text{ N/m}
\]

The eqn of motion for \( t > 0 \) becomes

\[
c \ddot{x} + kx = 0 \quad (E1)
\]

The solution of Eq. (E1) is given by

\[
x(t) = A \cdot e^{-\frac{k}{c} t} = A \cdot e^{-\frac{5000}{c} t}
\]

At \( t = 0 \), \( x(t) = 0.1 \) and hence

\[
0.1 = A \cdot e^{0} \quad \text{or} \quad A = 0.1
\]

\[
\therefore x(t) = 0.1 e^{-\frac{5000}{c} t}; \quad t > 0 \quad (E2)
\]

Using \( x(t = 10) = 0.01 \text{ m} \) in (E2),

\[
0.01 = 0.1 e^{-\frac{(5000/c)10}{c}} \quad \text{or} \quad e^{-\frac{(50,000)}{c}} = 0.1
\]

i.e., \( -\frac{50,000}{c} = \ln 0.1 = -2.3026 \)

Hence \( c = 21714.7 \text{ N-s/m} \)
\[ m \ddot{v} = F - D - mg \]

\[ 1000 \ddot{v} = 50000 - 2000v - 1000(9.81) \]

\[ 1000 \ddot{v} + 2000v = 49190 \]

\[ 0.5 \ddot{v} + v = 20.095 \quad (E_1) \]

**Solution of Eq. (E_1) with \( v(0) = 0 \):**

\[ v(t) = 20.095 \left( 1 - e^{-0.5t} \right) \]

**or**

\[ \frac{dx}{dt} (t) = 20.095 \left( 1 - e^{-2t} \right) \quad (E_2) \]

**Integration of Eq. (E_2) gives**

\[ x(t) = 20.095t - 20.095 \left( \frac{1}{-2} \right) e^{-2t} + C_1 \]

\[ = 20.095t + 10.0475 \cdot e^{-2t} + C_1 \]

\[ x(0) = 0 \]

\[ \Rightarrow 0 = 10.0475 e^{0} + C_1 \]

\[ \Rightarrow C_1 = -10.0475 \]

\[ \therefore x(t) = 20.095t + 10.0475 e^{-2t} - 10.0475 \]

2-96
Let $m_{\text{eff}}$ = effective part of mass of beam (m) at middle. Thus vibratory inertia force at middle is due to $(M + m_{\text{eff}})$. Assume a deflection shape:

$$y(x, t) = Y(x) \cos (\omega_n t - \phi)$$

where $Y(x)$ = static deflection shape due to load at middle given by:
\[ Y(x) = Y_0 \left( 3 \frac{x}{\ell} - 4 \frac{x^3}{\ell^3} \right) ; \quad 0 \leq x \leq \frac{\ell}{2} \]

where \( Y_0 \) = maximum deflection of the beam at middle = \( \frac{F \ell^3}{48EI} \)

Maximum strain energy of beam = maximum work done by force \( F = \frac{1}{2} F Y_0 \).

Maximum kinetic energy due to distributed mass of beam:

\[
= 2 \left\{ \frac{1}{2} \frac{m}{\ell} \int_0^\ell \dot{Y}^2(x,t) \bigg|_{\text{max}} \, dx \right\} + \frac{1}{2} \left\{ \dot{Y}_{\text{max}} \right\}^2 M
\]

\[
= \frac{m \omega_n^2}{\ell} \int_0^\ell Y^2(x) \, dx + \frac{1}{2} \omega_n^2 Y_{\text{max}}^2 M
\]

\[
= \frac{m \omega_n^2}{\ell} \int_0^\ell \left( \frac{9 x^2}{\ell^2} + 16 \frac{x^6}{\ell^6} - 24 \frac{x^4}{\ell^4} \right) \, dx + \frac{1}{2} Y_0^2 M \omega_n^2
\]

\[
= \frac{m \omega_n^2}{\ell} \left[ \frac{9}{3} \frac{x^3}{\ell^3} + 16 \frac{x^7}{7 \ell^7} - 24 \frac{x^5}{5 \ell^5} \right] \bigg|_0^\ell + \frac{1}{2} Y_0^2 M \omega_n^2
\]

\[
= \frac{1}{2} Y_0^2 \omega_n^2 \left( \frac{17}{35} m + M \right)
\]

This shows that \( m_{\text{eff}} = \frac{17}{35} m = 0.4857 \, m \)

\[ \text{(2.107)} \]

For small angular rotation of bar PQ about P,

\[
\frac{1}{2} (k_{12})_{eq} \theta \ell_3^2 = \frac{1}{2} k_1 (\theta \ell_1)^2 + \frac{1}{2} k_2 (\theta \ell_2)^2
\]

\[
(k_{12})_{eq} = \frac{k_1 \ell_1^2 + k_2 \ell_2^2}{\ell_3^2}
\]

Since \((k_{12})_{eq}\) and \(k_3\) are in series,

\[
k_{eq} = \frac{(k_{12})_{eq} k_3}{(k_{12})_{eq} + k_3} = \frac{k_1 k_3 \ell_1^2 + k_2 k_3 \ell_2^2}{k_1 \ell_1^2 + k_2 \ell_2^2 + k_3 \ell_3^2}
\]

\[ T = \text{kinetic energy} = \frac{1}{2} m x^2 \quad \text{,} \quad U = \text{potential energy} = \frac{1}{2} k_{eq} x^2
\]

If \( x = X \cos \omega_n t \),

\[
T_{\text{max}} = \frac{1}{2} m \omega_n^2 X^2 \quad \text{,} \quad U_{\text{max}} = \frac{1}{2} k_{eq} X^2
\]
\[ T_{\text{max}} = U_{\text{max}} \quad \text{gives} \quad \omega_n = \sqrt{\frac{k_1 k_3 l_1^2 + k_2 k_3 l_2^2}{m(k_1 l_1^2 + k_2 l_2^2 + k_3 l_3^2)}} \]

When mass \( m \) moves by \( x \), spring \( k_1 \) deflects by \( x/4 \).

\[ T = \text{kinetic energy} = \frac{1}{2} m (\dot{x})^2 \]

\[ U = \text{potential energy} = 2 \left\{ \frac{1}{2} (2k) \left( \frac{x}{4} \right)^2 \right\} = \frac{1}{8} k x^2 \]

For harmonic motion,

\[ T_{\text{max}} = \frac{1}{2} m \omega_n^2 x^2, \quad U_{\text{max}} = \frac{1}{8} k x^2 \]

\[ T_{\text{max}} = U_{\text{max}} \quad \text{gives} \quad \omega_n = \sqrt{\frac{k}{4m}} \]

Refer to the figure of solution of problem 2.24.

\[ T = \frac{1}{2} m \ddot{x}^2, \quad U = \frac{1}{2} \left[ 2k_1 \left( x \cos 45^\circ \right)^2 + 2k_2 \left( x \cos 135^\circ \right)^2 \right] = \frac{1}{2} (k_1 + k_2) x^2 \]

For harmonic motion,

\[ T_{\text{max}} = \frac{1}{2} m \omega_n^2 x^2, \quad U_{\text{max}} = \frac{1}{2} (k_1 + k_2) x^2 \]

\[ T_{\text{max}} = U_{\text{max}} \quad \text{gives} \quad \omega_n = \sqrt{\frac{k_1 + k_2}{m}} \]

**Kinetic energy (K.E.)**

\[ \frac{1}{2} \text{m} \ddot{x}^2 \]

**Potential energy (P.E.)**

\[ \frac{1}{2} T_1 x + \frac{1}{2} T_2 x = \text{work done in displacing mass m by distance x against the total force (tension) of } T_1 + T_2. \]

\[ T_1 = \frac{x}{a} T, \quad T_2 = \frac{x}{b} T \]

From solution of problem 2.26

Max. K.E. = \( \frac{1}{2} m \omega_n^2 x^2 \), \quad Max. P.E. = \( \frac{1}{2} T \left( \frac{1}{a} + \frac{1}{b} \right) x^2 \)

Max. K.E. = Max. P.E. gives

\[ \omega_n = \sqrt{\frac{T (a+b)}{ma}} \]

\[ T = k \cdot E = \frac{1}{2} \mathcal{J}_A \dot{\theta}^2 = \frac{1}{2} (\mathcal{J}_A + m d^2) \dot{\theta}^2 = \frac{1}{12} \left( \frac{m l^2}{36} + m \frac{l^2}{36} \right) \dot{\theta}^2 \]

\[ = \frac{1}{2} (\frac{ml^2}{9}) \dot{\theta}^2 \]

\[ U = P \cdot E = mgd (1 - \cos \theta) + 2 \left( \frac{1}{2} k x_1^2 + \frac{1}{2} k x_2^2 \right) + \frac{1}{2} k \dot{\theta}^2 \]

with \( \cos \theta = 1 - \frac{1}{2} \dot{\theta}^2 \), \( x_1 = \frac{l}{3} \theta \) and \( x_2 = \frac{2l}{3} \theta \)

\[ 2-99 \]
U = mg \frac{1}{6} \theta^2 + \kappa \frac{l^2}{9} \theta^2 + \kappa \frac{4 l^2}{9} \theta^2 + \frac{1}{2} \kappa_t \theta^2

T_{\text{max}} = \frac{1}{2} \left( \frac{ml^2}{9} \right) \theta^2, \quad U_{\text{max}} = \frac{1}{2} \left( \frac{mg l}{6} \right) \theta^2 + \frac{1}{2} \left( \frac{10 k l^2}{9} \right) \theta^2 + \frac{1}{2} \kappa_t \theta^2

T_{\text{max}} = U_{\text{max}} \text{ gives}

\omega_n = \sqrt{\frac{m g l}{6} + \frac{10 k l^2}{9} + \kappa_t}{\frac{ml^2}{9}} = 45.1547 \frac{\text{rad}}{\text{sec}} \text{ for given data}

Refer to the figure in the solution of problem 2.76

T = \frac{1}{2} J_0 \dot{\theta}^2

U = \frac{1}{2} \kappa_t \theta^2 + \frac{1}{2} \kappa_1 (\dot{\theta}a)^2 + \frac{1}{2} \kappa_2 (\dot{\theta} l)^2

For \theta(t) = \Theta \cos \omega_n t,

T_{\text{max}} = \frac{1}{2} J_0 \omega_n^2 \Theta^2, \quad U_{\text{max}} = \frac{1}{2} \left( \kappa_t + \kappa_1 a^2 + \kappa_2 l^2 \right) \Theta^2

T_{\text{max}} = U_{\text{max}} \text{ gives}

\omega_n = \sqrt{\frac{\kappa_t + \kappa_1 a^2 + \kappa_2 l^2}{J_0}} = \sqrt{\frac{3}{m l^2}} \frac{\kappa_t + \kappa_1 a^2 + \kappa_2 l^2}{J_0}

since \quad J_0 = ml^2/3.

When prism is displaced by \( x \) from equilibrium position, the weight of oil displaced

= \rho g ab x = restoring force

Mass of prism = \( m = \rho_w ab h \)

Equation of motion:

\[ m \ddot{x} + \text{restoring force} = 0 \]

\[ \rho_w ab h \ddot{x} + \rho g ab x = 0 \]

\[ \omega_n = \sqrt{\frac{\rho g ab}{\rho_w ab h}} = \sqrt{\frac{\rho g}{\rho_w h}} \quad (E1) \]

Since \( \omega_n \) is independent of cross-section of the prism, \( \omega_n \) remains same even for a circular wooden prism.

\[ T = \frac{1}{2} m \dot{x}^2 + \frac{1}{2} J_0 \dot{\theta}^2 = \frac{1}{2} \left( m R^2 + \frac{1}{2} m R^2 \right) \dot{\theta}^2 \]

since \( x = R \theta \) and \( J_0 = \frac{1}{2} m R^2 \).

\[ U = \frac{1}{2} k_1 x_1^2 + \frac{1}{2} k_2 x_1^2 = \frac{1}{2} (k_1 + k_2) (R + a)^2 \theta^2 \]

2-100
where $x_1 = (R + a) \theta$. Using $\frac{d}{dt} (T + U) = 0$, we obtain
\[
\left( \frac{3}{2} m R^2 \right) \ddot{\theta} + (k_1 + k_2) (R + a)^2 \theta = 0
\]

Let $x(t)$ be measured from static equilibrium position of mass. $T =$ kinetic energy of the system:
\[
T = \frac{1}{2} m \dot{x}^2 + \frac{1}{2} J_0 \dot{\theta}^2 = \frac{1}{2} \left( m + \frac{J_0}{r^2} \right) \dot{x}^2
\]

since $\dot{\theta} = \frac{\dot{x}}{r} =$ angular velocity of pulley. $U =$ potential energy of the system:
\[
U = \frac{1}{2} k y^2 = \frac{1}{2} k (16 x^2)
\]

since $y = \theta (4 r) = 4 x =$ deflection of spring. $\frac{d}{dt} (T + U) = 0$ leads to:
\[
m \ddot{x} + \frac{J_0}{r^2} \dddot{x} + 16 k x = 0
\]

This gives the natural frequency:
\[
\omega_n = \sqrt{\frac{16 k r^2}{m r^2 + J_0}}
\]
Assume: No sliding of the cylinder.

Kinetic energy of the cylinder \((T)\) = sum of translational and rotational kinetic energies

\[
T = \frac{1}{2} m \dot{x}^2 + \frac{1}{2} J \dot{\theta}^2 \quad (E_1)
\]

Since the cylinder rolls without sliding,

\[
x = \theta R \quad \text{or} \quad \theta = \frac{x}{R} \quad (E_2)
\]

Using Eq. \((E_2)\), the kinetic energy can be expressed as

\[
T = \frac{1}{2} m \dot{x}^2 + \frac{1}{2} J \left(\frac{x}{R}\right)^2 = \frac{1}{2} \left(m + \frac{J}{R^2}\right) \dot{x}^2 \quad (E_3)
\]

\[
= \frac{1}{2} m \theta^2 R^2 + \frac{1}{2} J \dot{\theta}^2 = \frac{1}{2} \left(m R^2 + J\right) \dot{\theta}^2 \quad (E_4)
\]

The potential (or strain) energy, \(U\), due to the deflection of the spring is given by

\[
U = \frac{1}{2} k x^2 \quad (E_5)
\]

\[
= \frac{1}{2} k R^2 \theta^2 \quad (E_6)
\]

Total energy is constant since the damping is absent.

\[
T + U = c = \text{constant} \quad (E_7)
\]

Using Eqs. \((E_3)\) and \((E_5)\) in Eq. \((E_7)\), we obtain
\[
\frac{1}{2} \left( m + \frac{J}{R^2} \right) \dddot{x}^2 + \frac{1}{2} \kappa \dot{x}^2 = c \quad (E_8)
\]

Differentiating \( E_8 \) w.r.t. time gives

\[
\frac{1}{2} \left( m + \frac{J}{R^2} \right) (2 \dddot{x}) \dddot{x} + \frac{1}{2} \kappa (2 \dot{x} \dot{x}) = 0
\]

or

\[
\left[ (m + \frac{J}{R^2}) \dddot{x} + \kappa \dot{x} \right] \dot{x} = 0 \quad (E_9)
\]

Since \( \dot{x} \neq 0 \) for all \( t \),

\[
(m + \frac{J}{R^2}) \dddot{x} + \kappa \dot{x} = 0 \quad (E_{10})
\]

The natural frequency of vibration, from Eq. (E_{10}), is given by

\[
\omega_n = \sqrt{\frac{\kappa}{(m + \frac{J}{R^2})}} \quad (E_{11})
\]

Since the mass moment of inertia of a cylinder can be expressed as

\[
J = \frac{1}{2} m R^2 \quad (E_{12})
\]

Eqs. (E_{10}) and (E_{11}) become

\[
\frac{3}{2} m \dddot{x} + \kappa \dot{x} = 0 \quad (E_{13})
\]

\[
\omega_n = \sqrt{\frac{2 \kappa}{\frac{3}{2} m}} \quad (E_{14})
\]
Using Eqs. (E_4) and (E_6), the total energy of the system can be expressed as

\[ \frac{1}{2} (m R^2 + J) \dot{\theta}^2 + \frac{1}{2} \kappa R^2 \theta^2 = c = \text{constant} \quad (E_{15}) \]

Differentiation of Eq. (E_{15}) with respect to time gives

\[ \frac{1}{2} (m R^2 + J) (2 \ddot{\theta} \dot{\theta}) + \frac{1}{2} \kappa R^2 (2 \dot{\theta} \dot{\theta}) = 0 \quad (E_{16}) \]

\[ \left[ (m R^2 + J) \ddot{\theta} + \kappa R^2 \theta \right] \dot{\theta} = 0 \quad (E_{17}) \]

Since \( \dot{\theta} \neq 0 \) for all \( t \),

\[ (m R^2 + J) \ddot{\theta} + \kappa R^2 \theta = 0 \quad (E_{18}) \]

The natural frequency of vibration, from Eq. (E_{18}), is given by

\[ \omega_n = \sqrt{\frac{\kappa R^2}{m R^2 + J}} \quad (E_{19}) \]

Using Eq. (E_{12}), Eqs. (E_{18}) and (E_{19}) become

\[ \frac{3}{2} m R^2 \dddot{\theta} + \kappa R^2 \theta = 0 \quad (E_{20}) \]
\[ \omega_n = \sqrt{\frac{K R^2}{\frac{3}{2} m R^2}} = \sqrt{\frac{2K}{3m}} \quad (E_{21}) \]

It can be seen that the two equations of motion, Eqs. (E_{10}) and (E_{18}), lead to the same natural frequency \( \omega_n \) as shown in Eqs. (E_{14}) and (E_{21}).
Equation of motion: \( m \dddot{x} + c \ddot{x} + kx = 0 \) \hspace{1cm} (E.1)

(a) SI units (kg, N·m/s, N/m for m, c, k, respectively)
\( m = 2 \text{ kg} \), \( c = 800 \text{ N·s/m} \), \( k = 4000 \text{ N/m} \)
Eq. (E.1) becomes
\[
2 \dddot{x} + 800 \ddot{x} + 4000 x = 0 \hspace{1cm} (E.2)
\]

(b) British engineering units (slug, lb·s²/ft, lb·s/ft for m, c, k)
\( m: 1 \text{ kg} = 0.06852 \text{ slug} \)
\( c: 1 \text{ N·s/m} = 0.06852 \text{ lb·s/ft} \)
\( (\text{since } 0.4 \text{ lb·s/ft} = 5.837 \text{ N·s/m}) \)
\( k: 1 \text{ N/m} = 0.06852 \text{ lb·ft/ft} \)
Eq. (E.2) becomes
\[
2(0.06852) \dddot{x} + 800(0.06852) \ddot{x} + 4000(0.06852)x = 0 \hspace{1cm} (E.3)
\]
\[
2 \dddot{x} + 800 \ddot{x} + 4000 x = 0 \hspace{1cm} (E.2)
\]

(c) British absolute units (lb, poundal·s/ft, poundal/ft for m, c, k)
\( m: 1 \text{ kg} = 2.2045 \text{ lb} \)
\( c: 1 \text{ N·s/m} = \frac{7.233 \text{ poundal·s}}{3.281 \text{ ft}} = 2.2045 \text{ poundal·s/ft} \)
\( k: 1 \text{ N/m} = \frac{7.233 \text{ poundal}}{3.281 \text{ ft}} = 2.2045 \text{ poundal/ft} \)
Eq. (E.2) becomes
\[
2(2.2045) \dddot{x} + 800(2.2045) \ddot{x} + 4000(2.2045)x = 0 \hspace{1cm} (E.4)
\]
which can be seen to be same as Eq. (E.2).

(d) Metric engineering units (kg·s²/m, kg·s²/m, kg·s²/m for m, c, k)
\( m: 1 \text{ kg} = 0.10197 \text{ kg·s²/m} \)

2-106
\[ c: \frac{N-x}{m} = \left(\frac{1}{9.807}\right) \frac{kg - x}{1 m} = 0.10197 \frac{kg - x}{m} \]

\[ k: \frac{N}{m} = \left(\frac{1}{9.807}\right) \frac{kg}{1 m} = 0.10197 \frac{kg}{m} \]

Eg. (E.2) becomes
\[ 2(0.10197) \ddot{x} + 800(0.10197) \dot{x} + 4000(0.10197) x = 0 \]  \( (E.5) \)

which can be seen to be same as Eg. (E.2).

(e) Metric absolute or cgs system (gram, dyne-s/cm, dyne/cm for m, c and k)

\[ m: 1 \text{ kg} = 1000 \text{ grams} \]

\[ c: \frac{N-x}{m} = \frac{10^5 \text{ dyne}s}{10^2 \text{ cm}} = 1000 \frac{\text{dyne-s}}{\text{cm}} \]

\[ k: \frac{N}{m} = \frac{10^5 \text{dyne}}{10^2 \text{ cm}} = 1000 \frac{\text{dyne}}{\text{cm}} \]

Eg. (E.2) becomes
\[ 2(1000) \ddot{x} + 800(1000) \dot{x} + 4000(1000) x = 0 \]  \( (E.6) \)

which can be seen to be same as Eg. (E.2).

(f) US customary units (lb, lbf-s/ft, lbf/ft for m, c and k)

\[ m: 1 \text{ kg} = 0.6852 \text{ slug} = 0.06252 \text{ lb}f - s^2/ft \]

\[ = 2.204 \text{ lbf-s}/(32.2 \text{ ft}/s^2) \]

\[ c: \frac{N-x}{m} = \frac{0.2248 \text{ lb}f - s}{3.281 \text{ ft}} = 0.06852 \frac{\text{lb}f - s}{\text{ft}} \]

\[ k: \frac{N}{m} = 0.2248 \frac{\text{lb}f - s}{3.281 \text{ ft}} = 0.06852 \frac{\text{lb}f}{\text{ft}} \]

Eg. (E.2) becomes
\[ 2(0.06252) \ddot{x} + 800(0.06252) \dot{x} + 4000(0.06252) x = 0 \]  \( (E.7) \)

which can be identified to be same as Eg. (E.2).
2.118

\[ m = 5 \text{ kg}, \quad c = 500 \text{ N} \cdot \text{s/m}, \quad k = 5000 \text{ N/m} \]

**Undamped natural frequency:**

\[ \omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{5000}{5}} = 31.6228 \text{ rad/s} \]

**Critical damping constant:**

\[ c_c = 2 \sqrt{\frac{k}{m}} \]

\[ = 2 \sqrt{5000} (5) \]

\[ = 316.2278 \text{ N} \cdot \text{s/m} \]

**Damping ratio:**

\[ \zeta = \frac{c}{c_c} = \frac{500}{316.2278} = 1.5811 \]

Since it is overdamped, the system will not have damped frequency of vibration.

2.119

\[ m = 5 \text{ kg}, \quad c = 500 \text{ N} \cdot \text{s/m}, \quad k = 50,000 \text{ N/m} \]

**Undamped natural frequency:**

\[ \omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{50000}{5}} = 100 \text{ rad/s} \]

**Critical damping constant:**

\[ c_c = 2 \sqrt{\frac{k}{m}} = 2 \left( \frac{50000 \times 5}{2} \right)^{\frac{1}{2}} = 1000 \text{ N} \cdot \text{s/m} \]

**Damping ratio:**

\[ \zeta = \frac{c}{c_c} = \frac{500}{1000} = 0.5 \]

System is underdamped.

**Damped natural frequency:**

\[ \omega_d = \omega_n \sqrt{1 - \zeta^2} = 100 \sqrt{1 - (0.5)^2} \]

\[ = 86.6025 \text{ rad/s} \]

2-108
\[ m = 5 \text{ kg, } c = 1000 \text{ N-s/m, } k = 50000 \text{ N/m} \]

**Undamped natural frequency:**
\[
\omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{50000}{5}} = 100 \text{ rad/s}
\]

**Critical damping constant:**
\[
c_c = 2 \sqrt{k m} = 2 \sqrt{50000 \times (5)} = 1000 \text{ N-s/m}
\]

**Damping ratio:**
\[ \zeta = \frac{c}{c_c} = \frac{1000}{1000} = 1 \]

The system is critically damped.

\[
\omega_d = \omega_n \sqrt{1-\zeta^2} = 100 \sqrt{1-1^2} = 0
\]

Damped natural frequency is zero.
Damped single d.o.f. system:

\[ m = 10 \text{ kg}, \ k = 10,000 \text{ N/m}, \ \zeta = 0.1 \text{ (underdamped)} \]

\[ \omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{10,000}{10}} = 31.6228 \text{ rad/s} \]

Displacement of mass is given by Eq. (2.70f):

\[ x(t) = X e^{-\zeta \omega_n t} \cos(\omega_d t - \phi) \quad \text{(E.1)} \]

where

\[ \omega_d = \omega_n \sqrt{1 - \zeta^2} = 31.6228 \sqrt{1 - 0.01} = 31.4647 \text{ rad/s} \]

\[ X = \frac{\left( x_0^2 + \omega_n^2 \dot{x}_0^2 + 2 x_0 \dot{x}_0 \zeta \omega_n \right)^{\frac{1}{2}}}{\omega_d} \quad \text{(2.73)} \]

and

\[ \phi = \tan^{-1} \left( \frac{\dot{x}_0 + \zeta \omega_n x_0}{x_0 \omega_d} \right) \quad \text{(2.75)} \]

(a) \[ x_0 = 0.2 \text{ m}, \ \dot{x}_0 = 0 \]

\[ X = \frac{(0.2)^2 (31.6228)^2}{31.4647} = 0.2010 \text{ m} \]

\[ \phi = \tan^{-1} \left( \frac{0.1 (31.6228)(0.2)}{0.2 (31.4647)} \right) = \tan^{-1} (0.1005) \]

\[ = 5.7391^\circ \text{ or } 0.1002 \text{ rad} \]

\[ \therefore x(t) = 0.2010 \cos (31.4647t - 0.1002) \text{ m} \]

(b) \[ x_0 = -0.2, \ \dot{x}_0 = 0 \]

\[ X = \frac{(-0.2)^2 (31.6228)^2}{31.4647} = 0.2010 \text{ m} \]

\[ \phi = \tan^{-1} \left( \frac{0.1 (31.6228)(-0.2)}{(-0.2)(31.4647)} \right) = \tan^{-1} (0.1005) \]

\[ = 185.7391^\circ \text{ or } 3.2418 \text{ rad} \]

(since both numerator and denominator in Eq. (2.75) are negative, \( \phi \) lies in third quadrant)

\[ \therefore x(t) = 0.2010 \cos (31.4647t - 3.2418) \text{ m} \]
(c) \( x_0 = 0, \quad \dot{x}_0 = 0.2 \text{ m/s} \)

\[
X = \frac{\sqrt{\left(0.2\right)^2}}{31.4647} = 0.006356 \text{ m}
\]

\[
\phi = \tan^{-1}\left(\frac{0.2}{\omega}\right) = \tan (\infty) = 90^\circ \text{ or } 1.5708 \text{ rad}
\]

\[
\therefore x(t) = 0.006356 e^{-3.1623 t} \cos \left(31.4647 t - 1.5708\right) \text{ m}
\]
Damped single d.o.f. system:

\[ m = 10 \text{ kg}, \ k = 10,000 \text{ N/m}, \ \zeta = 1.0 \ (\text{critically damped}) \]

\[ \omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{10,000}{10}} = 31.6228 \text{ rad/s} \]

Displacement of mass given by Eq. (2.80):

\[ x(t) = \{x_0 + (\dot{x}_0 + \omega_n x_0) t\} \ e^{-\omega_n t} \]

(a) \[ x_0 = 0.2 \text{ m}, \ \dot{x}_0 = 0 \]

\[ x(t) = \{0.2 + 31.6228 (0.2) t\} \ e^{-31.6228 t} \]

\[ = \left(0.2 + 6.32456 t\right) \ e^{-31.6228 t} \text{ m} \]

(b) \[ x_0 = -0.2 \text{ m}, \ \dot{x}_0 = 0 \]

\[ x(t) = \{-0.2 + 31.6228 (-0.2) t\} \ e^{-31.6228 t} \]

\[ = -\left(0.2 + 6.32456 t\right) \ e^{-31.6228 t} \text{ m} \]

(c) \[ \dot{x}_0 = 0.2 \text{ m/s}, \ x_0 = 0 \]

\[ x(t) = \{0.2 t\} \ e^{-31.6228 t} \]

\[ = 0.2 t \ e^{-31.6228 t} \text{ m} \]

2-112
Single d.o.f. system:

\[ m = 10 \, \text{kg}, \quad k = 10000 \, \text{N/m}, \quad \xi = 2.0 \text{ (over damped)} \]

\[ \omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{10000}{10}} = 31.6228 \, \text{rad/s} \]

Displacement of mass given by Eq. (2.81):

\[ x(t) = C_1 e^{-\xi \omega_n t} + C_2 e^{(-\xi - \sqrt{\xi^2 - 1}) \omega_n t} \]

where

\[ C_1 = \frac{x_0 \omega_n (\xi + \sqrt{\xi^2 - 1}) + \dot{x}_0}{2 \omega_n \sqrt{\xi^2 - 1}} \]

\[ C_2 = \frac{-x_0 \omega_n (\xi - \sqrt{\xi^2 - 1}) - \dot{x}_0}{2 \omega_n \sqrt{\xi^2 - 1}} \]

(a) \[ x_0 = 0.2 \, \text{m}, \quad \dot{x}_0 = 0 \]

\[ C_1 = \frac{0.2 \left( 31.6228 \right) \left( 2 + \sqrt{3} \right)}{2 \left( 31.6228 \right) \sqrt{3}} = 0.2155 \]

\[ C_2 = \frac{-0.2 \left( 31.6228 \right) \left( 2 - \sqrt{3} \right)}{2 \left( 31.6228 \right) \sqrt{3}} = -0.01547 \]

\[ x(t) = 0.2155 e^{(-2 + \sqrt{3}) (31.6228) t} - 0.01547 e^{(-2 - \sqrt{3}) (31.6228) t} \]

\[ = 0.2155 e^{-8.4749 t} - 0.01547 e^{-118.0163 t} \, \text{m} \]

(b) \[ x_0 = -0.2 \, \text{m}, \quad \dot{x}_0 = 0 \]

\[ C_1 = \frac{-0.2 \left( 31.6228 \right) \left( 2 + \sqrt{3} \right)}{2 \left( 31.6228 \right) \sqrt{3}} = -0.2155 \]

\[ C_2 = \frac{0.2 \left( 31.6228 \right) \left( 2 - \sqrt{3} \right)}{2 \left( 31.6228 \right) \sqrt{3}} = 0.01547 \]
\[
x(t) = -0.2155 e^{(-2+\sqrt{3}) (31.6228 \ t)} + 0.01547 e^{(-2-\sqrt{3}) (31.6228 \ t)} - 8.4749 t - 18.0163 t \ m \\
\]

(C) \( x_0 = 0, \dot{x}_0 = 0.2 \ m/s \)

\[
C_1 = \frac{0.2}{2 (31.6228 \sqrt{3})} = 0.001826 \\
C_2 = \frac{-0.2}{2 (31.6228 \sqrt{3})} = -0.001826 \\
x(t) = 0.001826 \left\{ e^{(-2+\sqrt{3}) (31.6228 \ t)} - e^{(-2-\sqrt{3}) (31.6228 \ t)} \right\} \\
= 0.001826 \left\{ -8.4749 t - 118.0163 t \right\} \ m \\
\]

2-114
Torsional stiffness of the shaft of diameter \( d \) and length \( l \) is given by

\[ k_t = \frac{G I_0}{l} = \frac{G}{l} \frac{\pi}{32} d^4 \]  

(1)

Since the shafts on the two sides of the disk act as parallel torsional springs (because the torque on the disk is shared by the two torsional springs), the resultant spring constant is given by

\[ k_{teq} = k_{t1} + k_{t2} = \frac{G \pi d_1^4}{32 l_1} + \frac{G \pi d_2^4}{32 l_2} \]

\[ = \frac{G \pi d^4}{32} \left( \frac{1}{l_1} + \frac{1}{l_2} \right) \]

\[ = \frac{G \pi d^4}{32} \left( \frac{l_1 + l_2}{l_1 l_2} \right) \]  

(2)

Using \( l_1 = l_2 = \frac{1}{2} l \), Eq. (2) becomes

\[ k_{teq} = \frac{G \pi d^4}{32} \left( \frac{l}{2} + \frac{l}{2} \right) = \frac{G \pi d^4}{8 l} \]  

(3)

Natural frequency of the disk in torsional vibration is given by

\[ \omega_n = \sqrt{\frac{k_{teq}}{J}} = \sqrt{\frac{\pi G d^4}{8 l J}} \]
For pendulum, \[ \omega_n = \sqrt{\frac{g}{l}} \text{ in vacuum} = 0.5 \text{ Hz} = \pi \text{ rad/sec} \]

\[ l = \frac{g}{\omega_n^2} = \frac{9.81}{\pi^2} = 0.9840 \text{ m} \]

\[ \omega_d = \omega_n \sqrt{1 - \gamma^2} \text{ in viscous medium} = 0.45 \text{ Hz} = 0.45 \pi \text{ rad/sec} \]

\[ \gamma^2 = \frac{\omega_n^2 - \omega_d^2}{\omega_n^2} = \pi^2 \left( \frac{1 - 0.45}{\pi^2} \right) = 0.19 \]

\[ \gamma = 0.4359; \text{ System is under damped.} \]

Equation of motion: \[ mL^2 \ddot{\theta} + c_t \dot{\theta} + mgL \theta = 0 \]

\[ c_t = 2(mL^2) \omega_n = 2(1)(0.974)(\pi) = 6.2080 \]

\[ \gamma = \frac{c_t}{c_{ct}} = 0.4359 \]

Since \[ \omega_n = \frac{\omega_0}{\sqrt{l}} = \pi, \quad l = \frac{g}{\omega_n^2} = \frac{9.81}{\pi^2} = 0.9939 \text{ m} \]

\[ c_t = \gamma c_{ct} = 0.4359(2)(0.9939^2) = 2.7061 \text{ N-m-s/rad} \]

From Eq. (2.85),

\[ \ln \left( \frac{x_j}{x_j + 1} \right) = \ln (18) \Rightarrow \frac{2\pi \gamma}{\sqrt{1 - \gamma^2}} = 2.8904 \]

\[ \gamma = \frac{(2.8904)^2}{(2.8904)^2 + 4\pi^2} \]

\[ \gamma = 0.4179 \]

2-116
(a) If damping is doubled, \( \tilde{\gamma}_{\text{new}} = 0.8358 \)

\[
\ln \left( \frac{x(t)}{x_{\text{d}+1}} \right) = \frac{2\pi \tilde{\gamma}_{\text{new}}}{\sqrt{1 - \tilde{\gamma}_{\text{new}}^2}} = \frac{2\pi (0.8358)}{\sqrt{1 - (0.8358)^2}} = 9.5656
\]

\[
\therefore \frac{x(t)}{x_{\text{d}+1}} = 14.265 \cdot 362
\]

(b) If damping is halved, \( \tilde{\gamma} = 0.2090 \)

\[
\ln \left( \frac{x(t)}{x_{\text{d}+1}} \right) = \frac{2\pi \tilde{\gamma}_{\text{new}}}{\sqrt{1 - \tilde{\gamma}_{\text{new}}^2}} = \frac{2\pi (0.2090)}{\sqrt{1 - (0.2090)^2}} = 1.3428
\]

\[
\therefore \frac{x(t)}{x_{\text{d}+1}} = 3.8296
\]

\[x(t) = X e^{-\tilde{\gamma} \omega_n t} \sin \omega_d t \quad \text{where} \quad \omega_d = \sqrt{1 - \tilde{\gamma}^2} \omega_n\]

For maximum or minimum of \( x(t) \),

\[
\frac{dx}{dt} = X e^{-\tilde{\gamma} \omega_n t} \left( -\tilde{\gamma} \omega_n \sin \omega_d t + \omega_d \cos \omega_d t \right) = 0
\]

As \( e^{-\tilde{\gamma} \omega_n t} \neq 0 \) for finite \( t \),

\[-\tilde{\gamma} \omega_n \sin \omega_d t + \omega_d \cos \omega_d t = 0\]

i.e.

\[\tan \omega_d t = \frac{\sqrt{1 - \tilde{\gamma}^2}}{\tilde{\gamma}}\]

Using the relation

\[
\sin \omega_d t = \frac{\tan \omega_d t}{\sqrt{1 + \tan^2 \omega_d t}} = \pm \sqrt{1 - \tilde{\gamma}^2} \frac{\sqrt{1 - \tilde{\gamma}^2}}{\tilde{\gamma}} = \pm \sqrt{1 - \tilde{\gamma}^2}
\]

we obtain

\[
\sin \omega_d t = \sqrt{1 - \tilde{\gamma}^2}, \quad \cos \omega_d t = \tilde{\gamma}
\]

and

\[
\sin \omega_d t = -\sqrt{1 - \tilde{\gamma}^2}, \quad \cos \omega_d t = -\tilde{\gamma}
\]

\[
\frac{d^2x}{dt^2} = X e^{-\tilde{\gamma} \omega_n t} \left[ \tilde{\gamma}^2 \omega_n^2 \sin \omega_d t - 2 \tilde{\gamma} \omega_n \omega_d \cos \omega_d t - \omega_d^2 \sin \omega_d t \right]
\]

when \( \sin \omega_d t = \sqrt{1 - \tilde{\gamma}^2} \) and \( \cos \omega_d t = \tilde{\gamma} \),

\[
\frac{d^2x}{dt^2} = -X e^{-\tilde{\gamma} \omega_n t} \omega_n^2 \sqrt{1 - \tilde{\gamma}^2} < 0
\]

\[
\therefore \sin \omega_d t = \sqrt{1 - \tilde{\gamma}^2} \text{ corresponds to maximum of } x(t).
\]

when \( \sin \omega_d t = -\sqrt{1 - \tilde{\gamma}^2} \) and \( \cos \omega_d t = -\tilde{\gamma} \).
\[
\frac{d^2 x}{dt^2} = X e^{-\gamma \omega_n t} \omega_n^2 \sqrt{1-\gamma^2} > 0
\]
\[
\therefore \sin \omega_d t = -\sqrt{1-\gamma^2} \text{ corresponds to minimum of } x(t).
\]

Enveloping curves:

Let the curve passing through the maximum (or minimum) points be

\[x(t) = C e^{-\gamma \omega_n t}\]

For maximum points, \(t_{\text{max}} = \frac{\sin^{-1}(\sqrt{1-\gamma^2})}{\omega_d}\)

and

\[C e^{-\gamma \omega_n t_{\text{max}}} = X e^{-\gamma \omega_n t_{\text{max}}} \sin \omega_d t_{\text{max}}\]

i.e.

\[C = X \sqrt{1-\gamma^2}\]

\[\therefore x_1(t) = X \sqrt{1-\gamma^2} e^{-\gamma \omega_n t}\]

Similarly for minimum points, \(t_{\text{min}} = \frac{\sin^{-1}(-\sqrt{1-\gamma^2})}{\omega_d}\)

and

\[C e^{-\gamma \omega_n t_{\text{min}}} = X e^{-\gamma \omega_n t_{\text{min}}} \sin \omega_d t_{\text{min}}\]

i.e.

\[C = -X \sqrt{1-\gamma^2}\]

\[\therefore x_2(t) = -X \sqrt{1-\gamma^2} e^{-\gamma \omega_n t}\]

\[x(t) = \left[x_0 + (\dot{x}_0 + \omega_n x_0) t\right] e^{-\omega_n t}\]  \(\text{(E1)}\)

For \(x_0 > 0\), graph of \(E_0\) \((E_1)\) is shown for different \(x_0\).

we assume \(\dot{x}_0 > 0\) as it is the only case that gives a maximum.

For maximum of \(x(t)\),

\[\frac{dx}{dt} = e^{-\omega_n t} \left\{-(\dot{x}_0 + \omega_n x_0) \omega_n t + \dot{x}_0\right\} = 0\]

\[t_m = \frac{\dot{x}_0}{\omega_n (\dot{x}_0 + \omega_n x_0)} \quad \text{----- (E2)}\]

\[\text{2-118}\]
\[
\frac{d^2x}{dt^2} = -\varepsilon \omega_n t \left( 2 \omega_n \dot{x}_0 + \omega_n^2 x_0 - \omega_n^2 (\dot{x}_0 + \omega_n x_0) t \right) \tag{E_3}
\]
(E_2) and (E_3) give
\[
\frac{d^2x}{dt^2} \bigg|_{t=t_m} = -\omega_n t_m \left( 2 \omega_n \dot{x}_0 + \omega_n^2 x_0 - \omega_n^2 (\dot{x}_0 + \omega_n x_0) t_m \right)
= -\omega_n \left( \frac{\dot{x}_0}{\omega_n (\dot{x}_0 + \omega_n x_0)} \right) \left( \omega_n \dot{x}_0 + \omega_n^2 x_0 \right) \tag{E_4}
\]
For \( x_0 > 0 \) and \( \dot{x}_0 > 0 \), \( \frac{d^2x}{dt^2} \bigg|_{t=t_m} < 0 \)
Hence \( t_m \) given by Eq. (E_2) corresponds to a maximum of \( x(t) \).
\[
x \bigg|_{t=t_m} = \left( x_0 + \frac{\dot{x}_0}{\omega_n} \right) e^{-\omega_n t_m}
= \left( x_0 + \frac{\dot{x}_0}{\omega_n} \right) e^{-\left( \frac{\dot{x}_0}{\omega_n (\dot{x}_0 + \omega_n x_0)} \right)} \tag{E_5}
\]

Equation (2.92) can be expressed as
\[
\delta = \frac{1}{m} \ln \left( \frac{x_T}{x_m} \right)
\]
For half cycle, \( m = \frac{1}{2} \) and hence
\[
\delta = 2 \ln \left( \frac{x_0}{x_{\frac{1}{2}}} \right) = 2 \ln \left( \frac{1}{0.15^2} \right)
\]
Necessary damping ratio \( \zeta \) is
\[
\zeta = \frac{3.7942}{\sqrt{\frac{(2\pi)^2}{3.7942^2} + \frac{8^2}{3.7942^2}}} = \frac{3.7942}{\sqrt{4\pi^2 + 3.7942^2}} = 0.5169
\]
(a)
\[
\zeta = \frac{3}{4}, \quad \zeta = 0.3877, \quad \text{the overshoot can be determined by finding } \delta \text{ from Eq. (2.85)}:
\]
\[
\delta = \frac{2\pi \zeta}{\sqrt{1-\zeta^2}} = \frac{2\pi (0.3877)}{\sqrt{1-0.3877^2}} = 2.6427 = 2 \ln \left( \frac{x_0}{x_{\frac{1}{2}}} \right)
\]
\[
\ln \left( \frac{x_0}{x_{\frac{1}{2}}} \right) = 1.32135
\]
\[
x_{\frac{1}{2}} = \frac{x_0}{e^{1.32135}} = 0.266775 \quad x_0
\]
\( \therefore \) overshoot is 26.6775%.
(b)
\[
\zeta = \frac{5}{4}, \quad \zeta = 0.6461, \quad \delta \text{ is given by}
\]
\[
\delta = \frac{2\pi \zeta}{\sqrt{1-\zeta^2}} = \frac{2\pi (0.6461)}{\sqrt{1-(0.6461)^2}} = 5.3189 = 2 \ln \left( \frac{x_0}{x_{\frac{1}{2}}} \right)
\]

\[2-119\]
\[ \frac{x_0}{x_{i+1}} = 14.2888, \quad \frac{x_1}{x_{i+1}} = 0.0700 \]
\[ \therefore \text{overshoot} = 7\% \]

2.130
(i) (a) Viscous damping, (b) Coulomb damping.

(iii) (a) \( \tau_d = 0.2 \text{ sec}, f_d = 5 \text{ Hz}, \omega_d = 31.416 \text{ rad/sec} \)
(b) \( \tau_n = 0.2 \text{ sec}, f_n = 5 \text{ Hz}, \omega_n = 31.416 \text{ rad/sec} \)

(ii) (a) \( \frac{x_i}{x_{i+1}} = e^{\zeta \omega_n \tau_d} \)
\[ \ln \left( \frac{x_i}{x_{i+1}} \right) = \ln 2 = 0.6931 = \frac{2 \pi \zeta}{\sqrt{1 - \zeta^2}} \]
or \[ 39.890 \zeta^2 = 0.4804 \quad \text{or} \quad \zeta = 0.1096 \]
Since \( \omega_d = \omega_n \sqrt{1 - \zeta^2} \), we find
\[ \omega_n = \frac{\omega_d}{\sqrt{1 - \zeta^2}} = \frac{31.416}{\sqrt{0.98798}} = 31.6065 \text{ rad/sec} \]
\[ k = m \omega_n^2 = \left( \frac{500}{9.81} \right) (31.6065)^2 = 5.0916 \times 10^4 \text{ N/m} \]
\[ \zeta = \frac{c}{c_c} = \frac{c}{2 \tau_n \omega_n} \]
Hence \( c = 2 m \omega_n \zeta = 2 \left( \frac{500}{9.81} \right) (31.6065) (0.1096) = 353.1164 \text{ N-s/m} \)

(b) From Eq. (2.135):
\[ k = m \omega_n^2 = \left( \frac{500}{9.81} \right) (31.416)^2 = 5.0304 \times 10^4 \text{ N/m} \]
Using \( N = W = 500 \text{ N} \),
\[ \mu = \frac{0.002 k}{4 W} = \frac{(0.002) (5.0304 \times 10^4)}{4 (500)} = 0.0503 \]

2.131
(a) \( c_c = 2 \sqrt{k m} = 2 \sqrt{5000 \times 50} = 1000 \text{ N-s/m} \]
(b) \( c = \frac{c_c}{2} = 500 \text{ N-s/m} \)
\[ \omega_d = \omega_n \sqrt{1 - \zeta^2} = \sqrt{\frac{500}{50}} \sqrt{1 - \left( \frac{c}{c_c} \right)^2} = \sqrt{\frac{5000}{50}} \sqrt{1 - \left( \frac{1}{2} \right)^2} \]
\[ = 8.6603 \text{ rad/sec} \]
(c) From Eq. (2.85), \( s = \frac{2 \pi}{\omega_d} \left( \frac{c}{2m} \right) = \frac{2 \pi}{8.6603} \left( \frac{500}{2 \times 50} \right) \]
\[ = 3.6276 \]

2-120

© 2017 Pearson Education, Inc., Hoboken, NJ. All rights reserved. This material is protected under all copyright laws as they currently exist. No portion of this material may be reproduced, in any form or by any means, without permission in writing from the publisher.
To find the maximum of $x(t)$, we set the derivative of $x(t)$ with respect to time $t$ equal to zero. Using Eq. (2.70),

$$x(t) = X e^{-\zeta \omega_n t} \sin (\omega_d t - \phi)$$

$$\frac{dx(t)}{dt} = -X \zeta \omega_n e^{-\zeta \omega_n t} \sin (\omega_d t - \phi) + \omega_d X e^{-\zeta \omega_n t} \cos (\omega_d t - \phi) = 0$$ \hspace{1cm} (E1)

i.e.,

$$X e^{-\zeta \omega_n t} [-\zeta \omega_n \sin (\omega_d t - \phi) + \omega_d \cos (\omega_d t - \phi)] = 0$$ \hspace{1cm} (E2)

Since $X e^{-\zeta \omega_n t} \neq 0$,

we set the quantity inside the square brackets equal to zero. This yields

$$\tan (\omega_d t - \phi) = \frac{\omega_d}{\zeta \omega_n} = \frac{\sqrt{1 - \zeta^2}}{\omega_n}$$ \hspace{1cm} (E3)

or

$$\omega_d t - \phi = \tan^{-1} \left( \frac{\sqrt{1 - \zeta^2}}{\zeta} \right)$$ \hspace{1cm} (E4)

(a)

In the present case, $m = 2000$ kg, $x_0 = 0$, $v = \dot{x}_0 = 10$ m/s, $k = 80,000$ N/m and $c = 20,000$ N·s/m and hence

$$\omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{80,000}{2000}} = 6.3245 \text{ rad/s}, \hspace{0.5cm} c = 2 \sqrt{k \cdot m} = 2 \sqrt{(80,000)(2000)} = 25,298.221$$

N·s/m, $\zeta = c / c_d = 0.7906$, $\omega_d = \omega_n \sqrt{1 - \zeta^2} = (6.3245) \sqrt{1 - (0.7906)^2} = 3.8727 \text{ rad/s}$,

$$\tan^{-1} \left( \frac{\sqrt{1 - \zeta^2}}{\zeta} \right) = \tan^{-1} \left( \frac{\sqrt{1 - 0.7906^2}}{0.7906} \right) = \tan^{-1} (0.7745) = 0.6590 \text{ rad.}$$

For the given initial conditions, Eqs. (2.75) and (2.73) give

$$\phi = \tan^{-1} \left( \frac{10}{0} \right) = \tan^{-1} (\infty) = \frac{\pi}{2} = 1.5708 \text{ rad and } X = \frac{10}{3.8727} = 2.5822 \text{ m}$$

2-121
(b) Equation (E4) can be rewritten as

\[ 3.8727 \, t = \phi + 0.6590 = 1.5708 + 0.6590 = 2.2298 \]

which gives \( t = t_{\text{max}} \) as \( t_{\text{max}} = 0.5758 \) s.

(a) Using the value of \( t_{\text{max}} \), Eq. (2.70) gives the maximum displacement of the car after engaging the springs and damper as

\[ x(t_{\text{max}}) = x_{\text{max}} = 2.5822 \, e^{-0.7996 \left( 0.3245 \, 0.5758 \right)} \cos \left( 3.8727 \times 0.5758 - 1.5708 \right) \]

\[ = 2.5822 \, (0.0562) \cos (0.6591) = 2.5822 \, (0.0562) \cos (37.7635^\circ) \]

\[ = 0.1147 \, \text{m}. \]

*Note: The condition used in Eq. (E1) is also valid for the minimum of \( x(t) \). As such, the sufficiency condition for the maximum of \( x(t) \) is to be verified. This implies that the second derivative, \( \frac{d^2 x(t)}{dt^2} \) at \( t = t_{\text{max}} \), should be negative for maximum of \( x(t) \).*

\[ \omega_n = 200 \, \text{cycles/min} = 20.944 \, \text{rad/sec}, \quad \omega_d = 180 \, \text{cycles/min} = 18.8496 \, \text{rad/sec} \]

\[ J_0 = 0.2 \, \text{kg-m}^2 \]

Since \( \omega_d = \sqrt{1 - \gamma^2} \omega_n \),

\[ \gamma = \sqrt{1 - \left( \frac{\omega_d}{\omega_n} \right)^2} = \sqrt{1 - \left( \frac{18.8496}{20.944} \right)^2} = 0.4359 \]

\[ C_t = 2 \, J_0 \, \omega_n \gamma = 2(0.2)(20.944)(0.4359) = 3.6518 \, \text{N-m-s/rad} \]

Eq. (2.72) can be used to obtain \( \theta(t) \) for \( \dot{\theta}_0 = 0 \), \( \theta_0 = 2^\circ = 0.03491 \) rad and \( t = \tau_d = \frac{2\pi}{\omega_d} = 0.3333 \) sec,

\[ \theta(t) = e^{-\frac{t}{\omega_d}} \left\{ \cos \left( \frac{\tau_d \omega_n}{\omega_d} \right) + \frac{\tau_d \omega_n}{\omega_d} \sin \left( \frac{\tau_d \omega_n}{\omega_d} \right) \right\} \]

\[ = e^{-(0.4359)(20.944)(0.3333)} \left\{ \cos \left( \frac{18.8496 \times 0.3333}{18.8496} \right) \right\} + \frac{0.4359 \times 20.944}{18.8496} \sin \left( \frac{18.8496 \times 0.3333}{18.8496} \right) \]

\[ = 0.001665 \, \text{rad} = 0.09541^\circ \]

2.134

Assume that the bicycle and the boy fall as a rigid body by 5 cm at point A. Thus the mass \( m_{\text{eq}} \) will be subjected to an initial downward displacement of 5 cm \( (t = 0 \) assumed at point A):

\[ x_0 = 0.05 \, \text{m}, \quad \dot{x}_0 = 0 \]

\[ \omega_n = \sqrt{\frac{k_{\text{eq}}}{m_{\text{eq}}}} = \sqrt{\frac{50000 \times 9.81}{800}} = 24.7614 \, \text{rad/sec} \]
\[ c_c = 2 \, m \, \omega_h = 2 \left( \frac{800}{9.81} \right) = 267.5 \, \text{N} \cdot \text{s/m} \]
\[ \zeta = \frac{c}{c_c} = \frac{1000.0}{4038.5566} = 0.2476 \, \text{(underdamping)} \]
\[ \omega_d = \omega_h \sqrt{1 - \zeta^2} = 24.7614 \sqrt{1 - 0.2476^2} = 23.9905 \, \text{rad/sec} \]

Response of the system:
\[ x(t) = X \, e^{-\zeta \omega_h t} \sin (\omega_d t + \phi) \]
where \[ X = \left( x_0^2 + \left( \frac{\dot{x}_0 + \zeta \omega_h x_0}{\omega_d} \right)^2 \right)^{1/2} \]

\[ = \left( (0.05)^2 + \left( \frac{0.2476 \times 24.7614 \times 0.05}{23.9905} \right)^2 \right)^{1/2} = 0.051607 \, \text{m} \]
and \[ \phi = \tan^{-1} \left( \frac{x_0 \omega_d}{\dot{x}_0 + \zeta \omega_h x_0} \right) = \tan^{-1} \left( \frac{0.05 \times 23.9905}{0.2476 \times 24.7614 \times 0.05} \right) = 75.6645^\circ \]

Thus the displacement of the boy (positive downward) in vertical direction is given by
\[ x(t) = 0.051607 \, e^{-6.1309 t} \sin (23.9905 t + 75.6645^\circ) \, \text{m} \]

Reduction in amplitude of viscously damped free vibration in one cycle = 0.5 in.
\[ \frac{x_1}{x_2} = \frac{6.0}{5.5} = 1.0909; \quad \ln \frac{x_1}{x_2} = 0.08701 = \frac{2 \pi \zeta}{\sqrt{1 - \zeta^2}} \]
i.e., \[ 0.007571 \, (1 - \zeta^2) = 39.478602 \, \zeta^2 \, \text{or} \, \zeta = 0.013847 \]

\[ T_d = 0.2 \, \text{sec} = \frac{2 \pi}{\omega_d} \quad \omega_d = 31.416 \, \text{rad/sec} \]

From Eq. (2.92) \[ 8 = \frac{4 \pi}{50} \quad L_n 10 = 0.04605 \]
\[ \gamma = \frac{8}{\sqrt{(2 \pi)^2 + 8 \pi^2}} = \frac{0.04605}{\sqrt{0.007329}} = 0.007329 \]

When damping is neglected,
\[ \omega_n = \omega_d / \sqrt{1 - \zeta^2} = 31.417 \, \text{rad/sec} \quad \zeta_n = \frac{2 \pi}{\omega_n} = 0.19999 \, \text{sec} \]
Proportional decrease in period \[ \approx (0.2 - 0.19999) / 0.2 = 0.00005 \]

For critically damped system, Eq. (2.80) gives
\[ x(t) = \{ x_0 + (x_0 + \omega_n x_0) t \} e^{-\omega_n t} \quad \text{(E1)} \]
\[ \dot{x}(t) = e^{-\omega_n t} \left\{ \dot{x}_0 + x_0 \omega_n t - \omega_n^2 x_0 t \right\} \quad \text{(E2)} \]

2-123
Let \( t_m = \text{time at which } x = x_{\text{max}} \text{ and } \dot{x} = 0 \text{ occur.} \) 
Here \( x_0 = 0 \text{ and } \dot{x}_0 = \text{initial recoil velocity. By setting } \dot{x}(t) = 0, \) Eq. \((E_2)\) gives
\[
\frac{\dot{x}_0}{\omega_n (x_0 + \omega_n x_0)} = \frac{x_0}{\omega_n \dot{x}_0} = \frac{1}{\omega_n} \tag{E_3}
\]
With Eq. \((E_3)\) for \( t_m \) and \( x_0 = 0 , \) \((E_1)\) gives
\[
x_{\text{max}} = \dot{x}_0 \frac{t_m e^{-\omega_n t_m}}{\omega_n} = \frac{\dot{x}_0 e^{-1}}{\omega_n} \tag{E_4}
\]
Using \( x_{\text{max}} = 0.5 \text{ m and } \dot{x}_0 = 10 \text{ m/s, Eq. \((E_4)\) gives}
\[
\omega_n = \frac{\dot{x}_0}{(x_{\text{max}} e^{-1})} = \frac{10}{(0.5 \times 2.7183)} = 7.3575 \text{ rad/s}
\]
When mass of gun is 500 kg, stiffness of spring is
\[
k = \frac{\omega_n^2 m}{(7.3575)^2 (500)} = 27,066 \text{ N/m}
\]
Note: Other values of \( x_0 \) and \( m \) can also be used to find \( k. \) Finally, the stiffness corresponding to least cost can be chosen.

\[
k = 5000 \text{ N/m}, \quad c_c = 0.2 \text{ N-s/m} = 200 \text{ N-s/m}
\]
\[
m = 2 \text{ kg}
\]
\[
\omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{5000}{2}} = 50 \text{ rad/sec}
\]
Logarithmic decrement \( \delta = \frac{2 \pi \tau}{\sqrt{1 - \tau^2}} \]
\[
i.e., \quad \tau = \frac{c_c}{k} = 0.3033 \quad \text{and} \quad c = 0.3033 (0.2) = 60.66 \text{ N-s/m}
\]
Assuming \( x_0 = 0 \) and \( \dot{x}_0 = 1 \text{ m/s,} \)
\[
x(t) = e^{-\tau \omega_n t} \frac{x_0}{\omega_n \sqrt{1 - \tau^2}} \sin \sqrt{1 - \tau^2} \omega_n t
\]
For \( x_{\text{max}}, \omega_n t \approx \pi/2 \) and \( \sin \sqrt{1 - \tau^2} \omega_n t \approx 1 \)
\[
\therefore \quad x_{\text{max}} \approx e^{-0.3033 (\pi/2)} \frac{1}{50 \sqrt{1 - 0.3033^2}} (1) = 0.01303 \text{ m}
\]

For an overdamped system, Eq. \((2.81)\) gives
\[
x(t) = e^{-\tau \omega_n t} (C_1 e^{\omega_d t} + C_2 e^{-\omega_d t}) \tag{E_1}
\]
Using the relations \( e^{\pm x} = \cosh x \pm \sinh x \) \((E_2)\) can be rewritten as
\[
x(t) = e^{-\tau \omega_n t} (C_3 \cosh \omega_d t + C_4 \sinh \omega_d t) \tag{E_3}
\]
where \( C_3 = c_1 + c_2 \) and \( C_4 = c_1 - c_2 \).

Differentiating (E3),
\[
\dot{x}(t) = e^{-\tau \omega_n t} \left[ C_3 \omega_d \sinh \omega_d t + C_4 \omega_d \cosh \omega_d t \right]
- \tau \omega_n e^{-\tau \omega_n t} \left[ C_3 \cosh \omega_d t + C_4 \sinh \omega_d t \right]
\]
(E4)

Initial conditions \( x(t=0) = x_0 \) and \( \dot{x}(t=0) = \dot{x}_0 \) with (E3) and (E4) give
\[
C_3 = x_0, \quad C_4 = \left( \dot{x}_0 + \tau \omega_n x_0 \right) / \omega_d
\]
(E5)

Thus (E3) becomes
\[
x(t) = x_0 e^{-\tau \omega_n t} \left( \cosh \omega_d t + \frac{\tau \omega_n}{\omega_d} \sinh \omega_d t \right)
+ \frac{\dot{x}_0}{\omega_d} e^{-\tau \omega_n t} \sinh \omega_d t
\]
(E6)

(i) When \( \dot{x}_0 = 0 \), Eq. (E6) gives
\[
x(t) = x_0 e^{-\tau \omega_n t} \left( \cosh \omega_d t + \frac{\tau \omega_n}{\omega_d} \sinh \omega_d t \right)
\]
(E7)

since \( e^{-\tau \omega_n t} \), \( \cosh \omega_d t \), \( \frac{\tau \omega_n}{\omega_d} \), and \( \sinh \omega_d t \) do not change sign (always positive) and \( e^{-\tau \omega_n t} \) approaches zero with increasing \( t \), \( x(t) \) will not change sign.

(ii) when \( x_0 = 0 \), Eq. (E6) gives
\[
x(t) = \frac{\dot{x}_0}{\omega_d} e^{-\tau \omega_n t} \sinh \omega_d t
\]
(E8)

Here also, \( \omega_d \), \( e^{-\tau \omega_n t} \) and \( \sinh \omega_d t \) do not change sign (always positive) and \( e^{-\tau \omega_n t} \) approaches zero with increasing \( t \), \( x(t) \) will not change sign.

\[2.140\]

Newton's second law of motion:
\[
\sum F = m \ddot{x} = -kx - cx + F_f \quad \text{(1)}
\sum M = J_0 \ddot{\theta} = -F_f R \quad \text{(2)}
\]

where \( F_f \) = friction force.

Using \( J_0 = \frac{m R^2}{2} \) and \( \ddot{\theta} = \frac{\ddot{x}}{R} \), Eq. (2) gives
\[
F_f = -\frac{1}{2} \frac{R}{m} \left( m R^2 \right) \frac{\ddot{x}}{R} = -\frac{1}{2} m \dddot{x}
\]

Substitution of Eq. (3) into (1) yields:
\[ \frac{3}{2} m \ddot{x} + c \dot{x} + kx = 0 \quad (4) \]

The undamped natural frequency is: \[ \omega_n = \sqrt{\frac{2k}{3m}} \quad (5) \]

**2.141 Newton's second law of motion:** (measuring \( x \) from static equilibrium position of cylinder)

\[ \sum F = m \ddot{x} = -kx - c \dot{x} - kx + F_f \quad (1) \]
\[ \sum M = J_0 \dot{\theta} = -F_f R \quad (2) \]

where \( F_f = \) friction force. Using \( J_0 = \frac{1}{2} m R^2 \) and \( \dot{\theta} = \frac{\ddot{x}}{R} \), Eq. (2) gives

\[ F_f = -\frac{1}{2} m \ddot{x} \quad (3) \]

Substitution of Eq. (3) into (1) gives

\[ \frac{3}{2} m \ddot{x} + c \dot{x} + 2kx = 0 \quad (4) \]

Undamped natural frequency of the system:

\[ \omega_n = \sqrt{\frac{4k}{3m}} \quad (4) \]

**2.142**

Consider a small angular displacement of the bar \( \theta \) about its static equilibrium position. Newton's second law gives:

\[ \sum M = J_0 \ddot{\theta} = -k \left[ \frac{3}{4} \theta \right] - c \left[ \frac{3}{4} \theta \right] - 3k \left[ \frac{3}{4} \theta \right] - 3k \left[ \frac{3}{4} \theta \right] = 0 \]

i.e., \[ J_0 \ddot{\theta} + \frac{c \ell^2}{16} \dddot{\theta} + \frac{3}{4} k \ell^2 \theta = 0 \]

© 2017 Pearson Education, Inc., Hoboken, NJ. All rights reserved. This material is protected under all copyright laws as they currently exist. No portion of this material may be reproduced, in any form or by any means, without permission in writing from the publisher.
where \( J_0 = \frac{7}{48} m \ell^2 \). The undamped natural frequency of torsional vibration is given by:

\[
\omega_n = \sqrt{\frac{3 k \ell^2}{4 J_0}} = \sqrt{\frac{36 k}{7 m}}
\]

2.143 Let \( \delta x \) = virtual displacement given to cylinder. Virtual work done by various forces:

Inertia forces: \( \delta W_i = -(J_0 \ddot{\vartheta}) (\delta \vartheta) - (m \ddot{x}) (\delta x) = -(J_0 \ddot{\vartheta}) \left( \frac{\delta x}{R} \right) - (m \ddot{x}) \delta x \)

Spring force: \( \delta W_s = -(k x) \delta x \)

Damping force: \( \delta W_d = -(c \dot{x}) \delta x \)

By setting the sum of virtual works equal to zero, we obtain:

\[- \frac{J_0}{R} \left( \ddot{x} \right) - m \ddot{x} - k x - c \dot{x} = 0 \quad \text{or} \quad \frac{3}{2} m \dddot{x} + c \ddot{x} + k x = 0\]

2.144 Let \( \delta x \) = virtual displacement given to cylinder from its static equilibrium position. Virtual works done by various forces:

Inertia forces: \( \delta W_i = -(J_0 \ddot{\vartheta}) \delta \vartheta - (m \ddot{x}) \delta \vartheta = -(J_0 \ddot{\vartheta}) \left( \frac{\delta x}{R} \right) = -(m \ddot{x}) \delta x \)

Spring force: \( \delta W_s = -(k x) \delta x - (k x) \delta x = -2 k x \delta x \)

Damping force: \( \delta W_d = -(c \dot{x}) \delta x \)

By setting the sum of virtual works equal to zero, we find

\[- \frac{J_0}{R} \left( \dddot{x} \right) - m \dddot{x} - 2 k x - c \ddot{x} = 0 \]

Using \( J_0 = \frac{1}{2} m R^2 \), Eq. (1) can be rewritten as

\[
\frac{3}{2} m \dddot{x} + c \ddot{x} + 2 k x = 0
\]

2.145 See figure given in the solution of Problem 2.114. Let \( \delta \vartheta \) be virtual angular displacement given to the bar about its static equilibrium position. Virtual works done by various forces:

Inertia force: \( \delta W_i = -(J_0 \ddot{\vartheta}) \delta \vartheta \)

Spring forces:

\( \delta W_s = -(k \theta \frac{3 \ell}{4}) \left( \frac{3 \ell}{4} \delta \vartheta \right) - (3 k \theta \frac{\ell}{4}) \left( \frac{\ell}{4} \delta \vartheta \right) = - \left( \frac{3}{4} k \ell^2 \vartheta \right) \delta \vartheta \)

Damping force: \( \delta W_d = -(c \dot{\vartheta} \frac{\ell}{4}) \left( \frac{\ell}{4} \delta \vartheta \right) \)

By setting the sum of virtual works equal to zero, we get the equation of motion as:

\[ J_0 \ddot{\vartheta} + c \frac{\ell^2}{16} \dot{\vartheta} + \frac{3}{4} k \ell^2 \vartheta = 0 \]
See solution of Problem 2.93. When wooden prism is given a displacement x, equation of motion becomes: \( m \ddot{x} + \text{restoring force} = 0 \)
where \( m = \text{mass of prism} = 40 \text{ kg} \) and restoring force = weight of fluid displaced = \( \rho_0 g a b x = \rho_0 (9.81)(0.4)(0.6) x = 2.3544 \rho_0 x \) where \( \rho_0 \) is the density of the fluid. Thus the equation of motion becomes:

\[
40 \ddot{x} + 2.3544 \rho_0 x = 0
\]

Natural frequency = \( \omega_n = \sqrt{\frac{2.3544 \rho_0}{40}} \)

Since \( \tau_n = \frac{2 \pi}{\omega_n} = 0.5 \), we find

\[
\omega_n = \frac{2 \pi}{0.5} = 4 \pi = \sqrt{\frac{2.3544 \rho_0}{40}}
\]

Hence \( \rho_0 = 2682.8816 \text{ kg/m}^3 \).

Let \( x = \text{displacement of mass} \) and \( P = \text{tension in rope on the left of mass} \).

Equations of motion:

\[
\sum F = m \ddot{x} = -k x - P \quad \text{or} \quad P = -m \dddot{x} - k x \tag{1}
\]
\[
\sum M = J_0 \dddot{\theta} = P r_2 - c (\dot{\theta} r_1) r_1 \tag{2}
\]

Using Eq. (1) in (2), we obtain

\[-(m \dddot{x} + k x) r_2 - c \dot{\theta} r_1^2 = J_0 \dddot{\theta} \tag{3}\]

With \( x = \theta r_2 \), Eq. (3) can be written as:

\[(J_0 + m r_2^2) \dddot{\theta} + c \theta r_1^2 \dddot{\theta} + k r_2^2 \theta = 0 \tag{4}\]

For given data, Eq. (4) becomes:

\[5 + 10 (0.25)^2 \dddot{\theta} + 0.125 \theta + 2 (0.25)^2 \theta = 0\]

or

\[5.625 \dddot{\theta} + 0.125 \theta + 0.0625 k \theta = 0 \tag{5}\]

Since amplitude is reduced by 80\% in 10 cycles,

\[
\frac{x_1}{x_{11}} = \frac{1.0}{0.2} = 5 = e^{10 \times \omega_n r_d}
\]

\[
\ln \frac{x_1}{x_{11}} = \ln 5 = 1.6094 = 10 \times \omega_n r_d \tag{6}\]

Since the natural frequency (assumed to be undamped torsional vibration frequency) is 5 Hz, \( \omega_n = 2 \pi (5) = 31.416 \text{ rad/sec} \). Also

\[
\tau_d = \frac{1}{r_d} = \frac{2 \pi}{\omega_d} = \frac{2 \pi}{\omega_n \sqrt{1 - \zeta^2}} = \frac{0.2}{\sqrt{1 - \zeta^2}} \tag{7}\]

Eq. (6) gives

\[\tau_d = 1 \frac{1}{r_d} = 2 \pi = \frac{2 \pi}{\omega_n \sqrt{1 - \zeta^2}} = \frac{0.2}{\sqrt{1 - \zeta^2}} \tag{7}\]
2.148 Torque = $2 \times 10^{-3}$ N·m

(a) Angle = 50° = 80 divisions

For a torsional system, Eq. (2.84) gives

$$\frac{\theta_1}{\theta_2} = e^\frac{T \omega_n \tau_d}{2}$$

(E1)

(b) For one cycle, $\tau_d = 2$ sec and (E1) gives

$$\frac{80}{5} = 2 \omega_n \tau_d \quad \text{or} \quad \omega_n = \frac{1}{2} \ln(16) = 1.3863$$

(E2)

Since

$$\tau_d = \frac{2\pi}{\omega_n^2 - \frac{\omega_n^2}{4}}$$

$$\omega_n^2 = \frac{(2\pi)^2}{\tau_d} + \frac{\omega_n^2}{4} = \frac{4\pi^2}{4} + 1.3863^2 = 11.7915$$

(E3)

(i.e., $\omega_n = 3.4339$ rad/sec)

(d) Since angular displacement of rotor under applied torque

$= 50° = 0.8727$ rad,

$$\kappa_t = \text{torque/angular displacement} = \frac{2 \times 10^{-3}}{0.8727}$$

$$= 2.2917 \times 10^{-3} \text{ N·m/rad}$$

(E4)

(a) Mass moment of inertia of rotor is

$$J_0 = \frac{\kappa_t}{\omega_n^2} = 2.2917 \times 10^{-3} / 11.7915 = 1.9436 \times 10^{-4} \text{ N·m·s}^2$$

(E5)

(c) $c_t = 2 J_0 \omega_n$

(E6)

Eqs. (E2) and (E3) give

$$\tau = \frac{T \omega_n}{\omega_n^2} = \frac{1.3863}{3.4339} = 0.4037$$

Eqs. (E6) gives

$$c_t = 5.3887 \times 10^{-4} \text{ N·m·s/rad}.$$

2-129
\[ \begin{align*}
\omega_n &= \sqrt{\frac{k}{m}} = \sqrt{\frac{1000}{10}} = 10 \text{ rad/s} \\
\gamma &= \frac{c}{2m \omega_n} = \frac{150}{2(10)(10)} = 0.75 \\
\omega_d &= \omega_n \sqrt{1 - \gamma^2} = 10 \sqrt{1 - 0.75^2} = 0 \\
\omega_d &= 6.61438 \text{ rad/s (critically-damped)} (\text{under-damped})
\end{align*} \]

(a) Underdamped system: Response: Eq. (2.70)

\[ \chi(t) = \left\{ \chi_0 + \frac{\dot{\chi}_0 + \gamma \omega_n \chi_0}{\omega_d} \right\} e^{-\gamma t} \cos \left( \frac{\omega_d t}{2} \right) \]

Using \( \chi_0 = 0.1, \dot{\chi}_0 = 10, \gamma = 0.75, \omega_n = 10, \omega_d = 6.61438, \)

\[ \chi(t) = 1.62832 e^{-7.5 t} \cos (6.61438 t + 1.50935) \text{ m} \]

(b) Critically damped system: Response: Eq. (2.80)

\[ \chi(t) = \left\{ \chi_0 + (\dot{\chi}_0 + \omega_n \chi_0) t \right\} e^{-\omega_n t} \]

2-130
\[\begin{align*}
&= \{0.1 + (10 + 10 \times 0.1) t\} e^{-10t} \\
&= (0.1 + 11t) e^{-10t} \quad \text{(c) overdamped system: Response: Eq. (2.81)}
\end{align*}\]

Using \(\sqrt{3^2 - 1} = \sqrt{1.25^2 - 1} = 0.75\), we obtain

\[\begin{align*}
C_1 &= \frac{\kappa_0 \omega_n \{5 + \sqrt{5^2 - 1}\} + \kappa_0}{2 \omega_n \sqrt{5^2 - 1}} \\
&= \frac{0.1 (10) \{1.25 + 0.75\} + 10}{2 (10) (0.75)} = 0.8 \\
C_2 &= \frac{-\kappa_0 \omega_n \{5 - \sqrt{5^2 - 1}\} - \kappa_0}{2 \omega_n \sqrt{5^2 - 1}} \\
&= \frac{-0.1 (10) \{1.25 - 0.75\} - 10}{2 (10) (0.75)} = -0.7
\end{align*}\]

Eq. (2.81) gives

\[\begin{align*}
\chi(t) &= C_1 e^{(-5 + \sqrt{5^2 - 1}) \omega_n t} + C_2 e^{(-5 - \sqrt{5^2 - 1}) \omega_n t} \\
&= 0.8 e^{(-1.25 + 0.75) (10) t} - 0.7 e^{(-1.25 - 0.75) (10) t} \\
&= 0.8 e^{-5t} - 0.7 e^{-20t} \quad \text{m}
\end{align*}\]

2.151 Energy dissipated in a cycle of motion,
\[\chi(t) = X \sin \omega_d t, \text{ is given by}\]
\[\Delta W = \pi c \omega_d X^2 \quad \text{(E.1)}\]

(a) \(\omega_n = \sqrt{\frac{K}{m}} = \sqrt{\frac{1000}{10}} = 10 \text{ rad/s}\)

\[\begin{align*}
\alpha &= \frac{c}{2m \omega_n} = \frac{50}{2 (10) (10)} = 0.25 \\
\omega_d &= \omega_n \sqrt{1 - \alpha^2} = 10 \sqrt{1 - 0.25^2} = 9.682458 \text{ rad/s}
\end{align*}\]

For \(X = 0.2 \text{ m}, \text{ Eq. (E.1) gives}\]
\[ \Delta W = \pi (50) (9.682458)(0.2^2) = 60.83682 \text{ Joules} \]

(b) \( \omega_n = \sqrt{\frac{k}{m}} = 10 \text{ rad/s} \)

\[ \gamma = \frac{c}{2m \omega_n} = \frac{150}{2(10)(10)} = 0.75 \]

\[ \omega_d = \omega_n \sqrt{1 - \gamma^2} = 10 \sqrt{1 - 0.75^2} = 6.614378 \text{ rad/s} \]

For \( x = 0.2 \text{ m}, \text{ Eq. (E.1) gives} \)

\[ \Delta W = \pi (150) (6.614378)(0.2^2) = 124.678385 \text{ Joules} \]
2.152

Equation of motion:

\[ 100 \ddot{x} + 500 \dot{x} + 10000 x + 400 x^3 = 0 \]

(a) Static equilibrium position is given by \( x = x_0 \) so that, for the nonlinear spring,

\[ 10000 x_0 + 400 x_0^3 = mg = 100 (9.81) = 981 \]

The value of \( x_0 \) is given by the root of

\[ 400 x_0^3 + 10000 x_0 - 981 = 0 \]

(Roots from MATLAB:

\( x_0 = 0.0981 \text{ m} \); other roots: \(-0.0490 \pm 5.0007 i)\)

(b) Linearized spring constant about the static equilibrium position \( x_0 = 0.0981 \text{ m} \) can be found as follows:

\[ F(x) = 400 x^2 + 10000 x \]

\[ K_{linear} = \left. \frac{dF}{dx} \right|_{x = x_0} = 1200 x_0^2 + 10000 \]

\[ = 1200 (0.0981)^2 + 10000 \]

\[ = 10011.5483 \text{ N/m} \]

Linearized equation of motion:

\[ 100 \ddot{x} + 500 \dot{x} + 10011.5483 x = 0 \]

(c) Natural frequency of vibration for small displacements:

\[ \omega_n = \left( \frac{10011.5483}{100} \right)^{\frac{1}{2}} = 10.0058 \text{ rad/s} \]
(a) Static equilibrium position is given by $x = x_0$ such that

$$-400 x_0^3 + 10000 x_0 = m g = 100 (9.81) = 981$$

or

$$-400 x_0^3 + 10000 x_0 - 981 = 0 \quad (1)$$

Roots of Eq. (1) are: (from MATLAB)

$x_0 = 0.0981$; other roots: $4.8502; -5.0483$

(b) Using the smallest positive root of Eq. (1) as the static equilibrium position, $x_0 = 0.0981$ m, the linearized spring constant about $x_0$ can be found as follows:

$$F(x) = -400 x_0^3 + 10000 x$$

$$K_{\text{linear}} = \frac{dF}{dx} \bigg|_{x_0} = -1200 x_0^2 + 10000$$

$$= 9988.4517 \text{ N/m}$$

Linearized equation of motion:

$$100 \ddot{x} + 500 \dot{x} + 9988.4517 x = 0 \quad (2)$$

(c) Natural frequency of vibration for small displacements:

$$\omega_n = \left( \frac{9988.4517}{100} \right)^{\frac{1}{2}} = 9.9942 \text{ rad/s}$$

2-134
Equation of motion:
\[ J_0 \ddot{\theta} + C_t \dot{\theta} + k_t \theta = 0 \]

with \( J_0 = 25 \text{ kg-m}^2 \) and \( k_t = 100 \text{ N-m/rad} \).

For critical damping, Eq. (2.105) gives
\[ c = C_c = 2 \sqrt{J_0 k_t} = 2 \sqrt{25(100)} \]
\[ = 100 \text{ N-m-s/rad}. \]
(a) \[ 2 \ddot{x} + 8 \dot{x} + 16x = 0 \]
\[ m = 2, \ c = 8, \ k = 16 \]
\[ x(0) = 0, \ \dot{x}(0) = 1 \]
\[ C_c = 2 \sqrt{km} = 2 \sqrt{16 \times 2} = 11.3137 \]
Since \( c < C_c \), system is underdamped.
\[ \zeta = \frac{c}{C_c} = \frac{8}{11.3137} = 0.7071 \]
\[ \omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{16}{2}} = 2.8284 \text{ rad/s} \]
\[ \omega_d = \omega_n \sqrt{1 - \zeta^2} = 2.8284 \sqrt{1 - 0.7071^2} = 2.0 \text{ rad/s} \]
Eq. (2.72) gives the solution:
\[ x(t) = e^{-\zeta \omega_n t} \left( x_0 \cos \omega_d t + \frac{x_0 + \zeta \omega_n x_0}{\omega_d} \sin \omega_d t \right) \]
\[ = e^{-0.7071 \times 2.8284 \times t} \left\{ 0 + \frac{1}{2} \sin 2t \right\} \]
\[ = \frac{1}{2} e^{-2t} \sin 2t \]

(b) \[ 3 \ddot{x} + 12 \dot{x} + 9x = 0 \]
\[ m = 3, \ c = 12, \ k = 9 \]
\[ x(0) = 0, \ \dot{x}(0) = 1 \]
\[ C_c = 2 \sqrt{km} = 2 \sqrt{9 \times 3} = 10.3923 \]
Since \( c > C_c \), system is overdamped.
\[ \zeta = \frac{c}{C_c} = \frac{12}{10.3922} = 1.1547 \]
\[ \omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{9}{3}} = 1.7320 \]
Solution is given by Eq. (2.81):
\[ C_1 = \frac{x_0 \omega_n \left( \zeta + \sqrt{\zeta^2 - 1} \right) + \dot{x}_0}{2 \omega_n \sqrt{\zeta^2 - 1}} \]

2-136
\[
\frac{1}{2 (1.7320) \sqrt{1.1547^2 - 1}} = 0.5
\]

\[C_2 = \frac{-x_0 \sqrt{5 - \sqrt{5^2 - 1}} - \dot{x}_0}{2 \omega_n \sqrt{5^2 - 1}} = -\frac{1}{2} = -0.5\]

Solution is:
\[x(t) = C_1 e^{(-5 + \sqrt{5^2 - 1}) \omega_n t} + C_2 e^{(-5 - \sqrt{5^2 - 1}) \omega_n t}\]
\[= 0.5 e^{-t} - 0.5 e^{-3t}\]

Since
\[-5 \pm \sqrt{5^2 - 1} = -1.1547 \pm \sqrt{1.1547^2 - 1}\]
\[= -1.1547 \pm 0.5773\]
\[= -1.732 \pm 0.5774\]

(c) \[2 \ddot{x} + 8 \dot{x} + 8x = 0\]
\[m = 2, \ c = 8, \ k = 8; \ x(0) = 0, \ \dot{x}(0) = 1\]
\[s = \frac{c}{2m} = \frac{8}{2 \sqrt[2]{\sqrt{8(2)}}} = 1\]

System is critically damped.
\[\omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{8}{2}} = 2 \text{ rad/s}\]

Solution is given by Eq. (2.80):
\[x(t) = \left\{x_0 + (\dot{x}_0 + \omega_n x_0) t\right\} e^{-\omega_n t}\]
\[= \left\{0 + (1 + 0) t\right\} e^{-2t}\]
\[= t e^{-2t}\]

2-137
(a) \[ 2 \ddot{x} + 8 \dot{x} + 16x = 0; \quad m = 2, \ c = 8, \ k = 16 \]
\[ x(0) = 1, \ \dot{x}(0) = 0 \]
\[ c_c = 2\sqrt{km} = 2\sqrt{16(2)} = 11.3137 \]
Since \( c < c_c \), system is underdamped
\[ \gamma = \frac{c}{c_c} = 0.7071, \ \omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{16}{2}} = 2.8284 \]
\[ \omega_d = \sqrt{1 - \gamma^2} \omega_n = 2.0 \]
Solution is given by Eq. (2.72):
\[ x(t) = e^{-\gamma \omega_n t} \left\{ x_0 \cos \omega_d t + \frac{\dot{x}_0 + \gamma \omega_n x_0}{\omega_d} \sin \omega_d t \right\} \]
\[ = e^{-0.7071(2.8284)t} \left\{ \cos 2t \left( \frac{2t}{2} + \frac{0.7071[2.8284](1)}{2} \sin 2t \right) \right\} \]
\[ = e^{-2t} \left( \cos 2t + \sin 2t \right) \]

(b) \[ 3 \ddot{x} + 12 \dot{x} + 9x = 0; \quad m = 3, \ c = 12, \ k = 9 \]
\[ x(0) = 1, \ \dot{x}(0) = 0 \]
\[ c_c = 2\sqrt{km} = 2\sqrt{9(3)} = 10.3923 \]
Since \( c > c_c \), system is overdamped
\[ \gamma = \frac{c}{c_c} = \frac{12}{10.3923} = 1.1547 \]
\[ \omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{9}{3}} = 1.7320 \]
\[ \sqrt{\gamma^2 - 1} = \sqrt{1.1547^2 - 1} = 0.5773 \]

2-138
\[ C_1 = \frac{x_0 \omega_n (5 + \sqrt{5^2 - 1})}{2 \omega_n \sqrt{5^2 - 1}} = \frac{1}{2} \left( \frac{1.7320}{1.7320} \right) \left( 1.1547 + 0.5773 \right) = 1.5 \]

\[ C_2 = -\frac{x_0 \omega_n (5 - \sqrt{5^2 - 1})}{2 \omega_n \sqrt{5^2 - 1}} = -\frac{1}{2} \left( \frac{1.7320}{1.7320} \right) \left( 1.1547 - 0.5773 \right) = -0.5 \]

Solution is:

\[ x(t) = 1.5 e^{(-5 + \sqrt{5^2 - 1}) \omega_n t} - 0.5 e^{(-5 - \sqrt{5^2 - 1}) \omega_n t} \]

\[ = 1.5 e^{-0.5774(1.732) t} - 1.732(1.732) t} - 0.5 e^{-0.5 e^{-3t}} \]

\[ = 1.5 e^{-t} - 0.5 e^{-3t} \]

(c) \[ 2x'' + 8x' + 8x = 0 \]

\[ m = 2, \ c = 8, \ \beta = 8 \]

\[ x(0) = 1, \ \dot{x}(0) = 0 \]

\[ c_c = 2\sqrt{km} = 2\sqrt{8(2)} = 8 \]

Since \( c = c_c \), system is critically damped.

\[ \xi = \frac{c}{c_c} = \frac{8}{8} = 1 \]

\[ \omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{8}{2}} = 2 \]

Solution is given by Eq. (2.80):

\[ x(t) = \{ x_0 + (\dot{x}_0 + \omega_n x_0) t \} e^{-\omega_n t} \]

\[ = \{ 1 + (0 + 2 \times 1) t \} e^{-2t} \]

\[ = (1 + 2t) e^{-2t} \]
(a) \(2 \ddot{x} + 8 \dot{x} + 16 x = 0\)

\[m = 2, \ c = 8, \ k = 16; \ x(0) = 1, \ \dot{x}(0) = -1\]

\[c_c = 2 \sqrt{k m} = 2 \sqrt{16(2)} = 11.3137\]

Since \(c < c_c\), system is underdamped.

\[\xi = \frac{c}{c_c} = \frac{8}{11.3137} = 0.7071\]

\[\omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{16}{2}} = 2.8284\]

\[\omega_d = \sqrt{1 - \xi^2} \omega_n = 2.0\]

Eq. (2.72) gives the solution as

\[x(t) = e^{-\xi \omega_n t} \left\{ x_0 \cos \omega_d t + \frac{x_0 + \xi \omega_n \omega_d}{\omega_d} \sin \omega_d t \right\}
= e^{-0.7071(2.8284)t} \left\{ \cos \omega_d t
+ \frac{-1 + 0.7071(2.8284)}{2} \sin \omega_d t \right\}
= e^{-2t} (\cos 2t + \frac{1}{2} \sin 2t)\]

(b)

\[3 \ddot{x} + 12 \dot{x} + 9 x = 0, \quad m = 3, \ c = 12, \ k = 9\]

\[x(0) = 1, \ \dot{x}(0) = -1\]

\[c_c = 2 \sqrt{k m} = 2 \sqrt{9(3)} = 2(5.1961) = 10.3923\]

Since \(c > c_c\), system is over-damped.

\[\xi = \frac{c}{c_c} = \frac{12}{10.3923} = 1.1547\]

\[\omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{9}{3}} = 1.732\]
\[
\sqrt{5^2-1} = \sqrt{1.1547^2-1} = 0.5773
\]
\[
5 + \sqrt{5^2-1} = 1.732
\]
\[
5 - \sqrt{5^2-1} = 0.5774
\]
\[
C_1 = \frac{(1)(\omega_n(1.732) - 1)}{2\omega_n(0.5773)} = \frac{2}{2} = 1
\]
\[
C_2 = \frac{-1+1}{2\omega_n(0.5773)} = 0
\]

Solution given by Eq. (2.81):
\[
x(t) = C_1 e^{(-5+\sqrt{5^2-1})\omega_n t} + C_2 e^{(-5-\sqrt{5^2-1})\omega_n t}
\]
\[
= e^{-0.5774(1.732)t} + e^{-t}
\]

(c) \[
2\ddot{x} + 8\dot{x} + 8x = 0; \quad m = 2, \; c = 8, \; k = 8
\]
\[
x(0) = 1, \quad \dot{x}(0) = -1
\]
\[
C_c = 2\sqrt{k/m} = 2\sqrt{(8)/(2)} = 8
\]
\[
\zeta = \frac{c}{C_c} = 1
\]
Hence system is critically damped.
\[
\omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{8}{2}} = 2
\]
solution is given by Eq. (2.80):

\[ x(t) = \left[ x_0 + \left( x_0 + \omega_n x_0 \right) t \right] e^{-\omega_n t} \]

\[ = \left[ 1 + (-1 + 2t) t \right] e^{-2t} \]

\[ = (1 + t) e^{-2t} \]
Frequency in air = \(120 \text{ cycles/min} = \frac{120}{60} (2\pi) = 4\pi \text{ rad/s} \)

Frequency in liquid = \(100 \text{ cycles/min} = \frac{100}{60} (2\pi) = 3.3333 \pi \text{ rad/s} \)

Assuming damping to be negligible in air, we have

\[ \omega_n = 4\pi = \sqrt{\frac{k}{m}} \Rightarrow k = (4\pi)^2 m = (4\pi)^2 \times 10 = 1579.1441 \text{ N/m} \]

If damping ratio in liquid is \(\xi\), and assuming underdamping, we have

\[ \omega_d = 3.3333 \pi = \omega_n \sqrt{1 - \xi^2} \]

or

\[ 1 - \xi^2 = \left(\frac{3.3333 \pi}{4\pi}\right)^2 = 0.6944 \]

or

\[ \xi = \left(1 - 0.6944\right)^{\frac{1}{2}} = 0.5528 \]

\[ \xi = \frac{c}{c_c} = \frac{c}{2m \omega_n} \]

or

\[ 0.5528 = \frac{c}{2(10)(4\pi)} \]

or

\[ c = 0.5528 \times 80\pi = 138.9341 \text{ N-s/m} \]
(a) \[ \ddot{x} + 2 \dot{x} + 9x = 0 \]
\[ m = 1, \ c = 2, \ k = 9; \ \ \ c_c = 2 \sqrt{k/m} = 2 \sqrt{9/1} = 6 \]
As \( c < c_c \), system is underdamped.
\[ \xi = \frac{c}{c_c} = \frac{2}{6} = 0.3333 \]
\[ \omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{9}{1}} = 3 \]
\[ \sqrt{1-\xi^2} = 0.9428; \ \ \ \omega_d = \omega_n \sqrt{1-\xi^2} = 2.8284 \]

Solution is given by Eq. (2.70):
\[ x(t) = X_0 e^{-0.3333(3)t} \cos (0.9428 x 3 t - \phi) \]
\[ = X_0 e^{-t} \cos (2.8284 t - \phi) \]
where \( X_0 \) and \( \phi \) depend on the initial conditions, as given by Eqs. (2.73) and (2.75), respectively.

Since the response (or solution) varies as \( e^{-t} \), we can apply the concept of the time constant \( \tau \) as the negative inverse of the exponential part. Hence the time constant is \( \tau = 1 \).

(b) \[ \ddot{x} + 8 \dot{x} + 9x = 0 \; \; \; m = 1, \ c = 8, \ k = 9 \]
\[ c_c = 2 \sqrt{k/m} = 2 \sqrt{9/1} = 6; \ \ \ \omega_n = \sqrt{\frac{k}{m}} = 3 \]
\[ \xi = \frac{c}{c_c} = \frac{8}{6} = 1.3333; \ \ \ \text{Hence} \ \ \ \text{the system is overdamped}. \]
\[ \sqrt{\xi^2 - 1} = \sqrt{1.3333^2 - 1} = 0.8819 \]
\[-5 - \sqrt{5^2 - 1} = -2.2152\]
\[-5 + \sqrt{5^2 - 1} = -0.4514\]

Solution is given by Eq. (2.81):

\[x(t) = C_1 e^{-0.4514(3)t} + C_2 e^{-2.2152(3)t}\]

\[= C_1 e^{-1.3512t} + C_2 e^{-6.6456t}\]

Since the response is given by the sum of two exponentially decaying functions, two time constants can be associated with the two parts as

\[\tau_1 = \frac{1}{1.3512} = 0.7384, \quad \tau_2 = \frac{1}{6.6456} = 0.1505\]

\[C\]

\[x + 6 \ddot{x} + 9x = 0; \quad m = 1, c = 6, k = 9\]

\[\omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{9}{1}} = 3\]

\[\zeta = \frac{c}{2\sqrt{km}} = 2\sqrt{9(1)} = 6; \quad \zeta = \frac{c}{\zeta} = 1\]

The system is critically damped. The solution is given by Eq. (2.80):

\[x(t) = \left\{ x_0 + (\dot{x}_0 + \omega_n x_0) t \right\} e^{-\omega_n t}\]

\[= \left\{ x_0 + (\dot{x}_0 + 3x_0) t \right\} e^{-3t}\]

since the solution decreases exponentially, the concept of time constant (\(\tau\)) can be applied to find

\[\tau = \frac{1}{3} = 0.3333\]

2-145
(a) Period of vibration = \( T \)

\[
\omega_n = \sqrt{\frac{k_t}{J}}
\]

\[
T = \frac{2\pi}{\omega_n} = 2\pi \sqrt{\frac{J}{k_t}}
\]

\[
\left( \frac{T}{2\pi} \right)^2 = \frac{J}{k_t}
\]

\[
J = k_t \left( \frac{T}{2\pi} \right)^2
\]

(b) \( T = 0.5 \text{ s} \)

\[
k_t = 5000 \text{ N} \cdot \text{m/rad}
\]

\[
J = 5000 \left( \frac{0.5}{2\pi} \right)^2 = 5000 \left( 0.006332 \right)
\]

\[
= 31.6627 \text{ N} \cdot \text{m-s}^2 = \text{kg} \cdot \text{m}^2
\]
2.161 Given: \( m = 2 \text{ kg} \), \( c = 3 \text{ N-s/m} \), \( k = 40 \text{ N/m} \)

Natural frequency: \( \Omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{40}{2}} = 4.4721 \text{ rad/s} \)

\( c = \text{critical damping} = 2 \sqrt{k_m} = 2 \sqrt{40 \times 2} = 17.8885 \text{ N-s/m} \)

\( \xi = \text{damping ratio} = \frac{c}{c_c} = \frac{3}{17.8885} = 0.1677 \)

Type of response in free vibration: damped oscillations

For critical damping, we need to add 14.8885 N-s/m to the existing value of \( c = 3 \text{ N-s/m} \).

2.162 Response of the system:

\[ x(t) = 0.05 e^{-10t} + 10.5 t e^{-10t} \text{ m} \]

This can be identified to correspond to critically damped system.

From the exponential terms, we find \( \omega_n = 10 \text{ rad/s} \)

From Eqs. (2.79), we find \( c_1 = 0.05 = x_0 \)

and \( c_2 = \dot{x}_0 + \omega_n x_0 \) or \( 10.5 = \dot{x}_0 + 10(0.05) \)

\( \therefore \dot{x}_0 = 0.05 \text{ m/s, } \ddot{x}_0 = 10.5 - 0.5 = 10 \text{ m/s}^2 \)

Damping constant (\( c \)):

\( c = c_c = 2 m \omega_n = 2 m (10) = 20 \text{ N-s/m} \)
2.163

characteristic Equations:

(a) \[ \lambda_{1,2} = -4 \pm 5i \]
\[
(\lambda + 4 + 5i)(\lambda + 4 - 5i) = (\lambda + 4)^2 - (5i)^2
= \lambda^2 + 8\lambda + 16 + 25 = \lambda^2 + 8\lambda + 41 = 0
\]
(b) \[ \lambda_{1,2} = 4 \pm 5i \]
\[
(\lambda - 4 - 5i)(\lambda - 4 + 5i) = (\lambda - 4)^2 - (5i)^2
= \lambda^2 - 16 - 8\lambda + 25 = \lambda^2 - 8\lambda + 41 = 0
\]
(c) \[ \lambda_{1,2} = -4, -5 \]
\[
(\lambda + 4)(\lambda + 5) = \lambda^2 + 9\lambda + 20
\]
(d) \[ \lambda_{1,2} = -4, -4 \]
\[
(\lambda + 4)(\lambda + 4) = \lambda^2 + 8\lambda + 16 = 0
\]

Undamped natural frequencies

(a) \[ m=1, \ c=8, \ k=41 \]
\[ \omega_n = \sqrt{\frac{k}{m}} = \sqrt{41} = 6.4031 \]
(b) \[ m=1, \ c=-8, \ k=41 \]
\[ \omega_n = \sqrt{\frac{k}{m}} = \sqrt{41} = 6.4031 \]
(c) \[ m=1, \ c=9, \ k=20 \]
\[ \omega_n = \sqrt{\frac{k}{m}} = \sqrt{20} = 4.4721 \]
(d) \[ m = 1, \ c = 8, \ k = 16 \]

\[ \omega_n = \sqrt{\frac{k}{m}} = \sqrt{16} = 4.0 \]

**Damping ratios**

\[ m \ddot{s} + c \dot{s} + k = 0 \]

\[ \zeta = \frac{c}{2m} \cdot \frac{1}{\omega_n} = \frac{c}{2 \sqrt{k/m}} \]

(a) \[ \zeta = \frac{8}{2 \sqrt{41(1)}} = \frac{8}{2 \sqrt{41}} = 0.6246 \]

(b) \[ \zeta = \frac{-8}{2 \sqrt{41(1)}} = \frac{-8}{2 \sqrt{41}} = -0.6246 \]

(c) \[ \zeta = \frac{9}{2 \sqrt{20(1)}} = \frac{9}{2 \sqrt{20}} = 0.9443 \]

(d) \[ \zeta = \frac{8}{2 \sqrt{16(1)}} = 1.0 \]

**Damped frequencies**

\[ \omega_d = \sqrt{1 - \zeta^2} \cdot \omega_n \quad \text{if} \quad \zeta < 1 \]

(a) \[ \omega_d = \sqrt{1 - 0.6246^2} \cdot (6.4031) = 5.0004 \]

(b) \[ \omega_d: \text{Not applicable} \]

(c) \[ \omega_d: \text{Not applicable} \]

(d) \[ \omega_d = 0 \]
\[ \tau = \frac{1}{5 \omega_n} = \frac{2\pi}{c} \]

(a) \[ \tau = \frac{1}{0.6246 (6.4031)} = 0.2500 \text{ (Underdamped)} \]

(b) \[ \tau = \frac{1}{-0.6246 (6.4031)} = -0.2500 \]

Not applicable; negative damping.
Response grows exponentially.

(c) \[ \tau = \frac{1}{1.0062 (4.4721)} = 0.2222 \text{ (Overshadowed)} \]

(d) \[ \tau = \frac{1}{1.0 (4.0)} = 0.25 \text{ (Undamped)} \]
(a)\[0 \; 5i\]
\[4 \; 0 \; \rightarrow \text{Re}\]
\[0 \; -5i\]
stable.
underdamped.
oscillatory response.

(b)\[5i \; 0\]
\[0 \; 4 \; \rightarrow \text{Re}\]
\[-5i \; 0\]
unstable.
Response grows exponentially.

(c)\[-5 \; 0 \; \rightarrow \text{Re}\]
\[-4 \; 0 \; 0\]
stable.
Response will not oscillate.

(d)\[-4, -4 \; 0 \; \rightarrow \text{Re}\]
\[-4 \; 0 \; 0\]
stable.
Response will not oscillate.
characteristic equation:
\[ s^2 + \omega s + b = 0 \]  

where
\[ a = \gamma m \]  
and
\[ b = k/m \]

Roots of Eq. (1):
\[ s_{1,2} = \frac{-a \pm \sqrt{a^2 - 4b}}{2} = -\frac{a}{2} \pm \sqrt{\left(\frac{a}{2}\right)^2 - b} \]  

and \( s_1 \) and \( s_2 \) are, in general, complex numbers.

Solution of Eq. (1) can be expressed as
\[ x(t) = c_1 e^{s_1 t} + c_2 e^{s_2 t} \]  

where \( c_1 \) and \( c_2 \) are constants.

- When \( s_1 \) and \( s_2 \) are both real and negative, the solution in Eq. (5) approaches zero asymptotically.
- If \( s_1 \) and \( s_2 \) are complex, the nature of solution is governed by the real part of the roots. If real part is negative, the solution in Eq. (5) is oscillatory and approaches zero as \( t \to \infty \).

The stability of the system in the s-plane is shown in Fig. a.
The stability of the system in the parameter space can be indicated as shown in Fig. b.

- When $a < 0$ and $b > 0$ (fourth quadrant), the curve $(\frac{a}{2})^2 - b = 0$ separates the quadrant into two regions. In the top part (above the parabola), the roots $\lambda_1$ and $\lambda_2$ will be complex conjugate with positive real part. Hence the motion will be diverging oscillations.

  In the bottom part (below the parabola curve), both $\lambda_1$ and $\lambda_2$ will be real with at least one positive root. Hence the motion diverges without oscillation.

- When $a > 0$ and $b > 0$ (first quadrant in Fig. b):

  The curve given by $(\frac{a}{2})^2 - b = 0$ (parabola) separates the quadrant into two regions. In the top region, $(\frac{a^2}{2} > b)$, $\lambda_1$ and $\lambda_2$ will be real and negative. Hence the motion decays without oscillations (aperiodic decay).

  In the region $(\frac{a^2}{2} < b)$, $\lambda_1$ and $\lambda_2$ will be complex conjugate with negative real part. Hence the response is oscillatory and decays as time increases.

  Along the boundary curve $(\frac{a^2}{2} - b = 0)$, the roots $\lambda_1$ and $\lambda_2$ will be identical with $\lambda_1 = \lambda_2 = \frac{a}{2}$. Hence the motion decays with time $t$. 

2-153
- When $a=0$ and $b>0$, the roots $s_1$ and $s_2$ will be pure imaginary complex conjugates. Hence the motion is oscillatory (harmonic) and stable.
- When $b<0$ (second and third quadrants), $s_1$ and $s_2$ will be positive and hence the response diverges with no oscillations; thus the motion is unstable.

![Diagram showing stability regions](image)

Figure b
characteristic equation:

\[ 2x^2 + cx + 18 = 0 \]  

(1)

Roots of Eq. (1):

\[ x_{1,2} = -c \pm \frac{\sqrt{c^2 - 144}}{4} \]  

(2)

At \( c = 0 \), the roots are given by \( x_{1,2} = \pm 3i \).

These roots are shown as dots in Fig. a. By increasing the value of \( c \), the roots can be found as shown in the following Table.

<table>
<thead>
<tr>
<th>( c )</th>
<th>( x_2 )</th>
<th>( x_1 )</th>
</tr>
</thead>
<tbody>
<tr>
<td>0</td>
<td>+3i</td>
<td>-3i</td>
</tr>
<tr>
<td>2</td>
<td>-0.5 + 2.96i</td>
<td>-0.5 - 2.96i</td>
</tr>
<tr>
<td>4</td>
<td>-1.0 + 2.83i</td>
<td>-1.0 - 2.83i</td>
</tr>
<tr>
<td>8</td>
<td>-2.0 + 2.24i</td>
<td>-2.0 - 2.24i</td>
</tr>
<tr>
<td>11</td>
<td>-2.75 + 1.20i</td>
<td>-2.75 - 1.20i</td>
</tr>
<tr>
<td>12</td>
<td>-3.0</td>
<td>-3.0</td>
</tr>
<tr>
<td>14</td>
<td>-3.5 + 1.80 = -1.70</td>
<td>-3.5 - 1.80 = -5.30</td>
</tr>
<tr>
<td>20</td>
<td>-5.0 + 4.0 = -1.0</td>
<td>-5.0 - 4.0 = -9.0</td>
</tr>
<tr>
<td>100</td>
<td>-25.0 + 24.82 = -0.18</td>
<td>-25.0 - 24.82 = -49.82</td>
</tr>
<tr>
<td>1000</td>
<td>-250 + 250 \approx 0</td>
<td>-250 - 250 \approx -500</td>
</tr>
</tbody>
</table>

Root locus is shown in Fig. a.
Problem 2.166 Root locus plot with variation of damping constant \( c \).

\[ s_1 = -\infty \quad s_1, s_2 \quad s_2 = -3i \]

\[ c = 0 \quad s_1 = 3i \quad s_2 = 0 \]

\[ c = \infty \quad c = 18 \]

Fig. (a)
Characteristic equation:

\[ 2s^2 + 12s + k = 0 \]  

(1)

Roots of Eq. (1):

\[ s_{1,2} = \frac{-12 \pm \sqrt{144 - 8k}}{4} \]  

(2)

or

\[ s_{1,2} = -3 \pm \sqrt{9 - \frac{1}{2}k} \]  

(3)

Since \( k \) cannot be negative, we vary \( k \) from 0 to \( \infty \). When \( k = 18 \), both \( s_1 \) and \( s_2 \) are real and equal to -3. In the range 0 < \( k < 18 \), both \( s_1 \) and \( s_2 \) will be real and negative.

When \( k = 0 \), \( s_1 = 0 \) and \( s_2 = -6 \). The variation of roots with increasing values of \( k \) is shown in the following Table and also in Fig. a.

<table>
<thead>
<tr>
<th>( k )</th>
<th>( s_1 )</th>
<th>( s_2 )</th>
</tr>
</thead>
<tbody>
<tr>
<td>0</td>
<td>0</td>
<td>-6.0</td>
</tr>
<tr>
<td>10</td>
<td>-1.0</td>
<td>-5.0</td>
</tr>
<tr>
<td>18</td>
<td>-3.0</td>
<td>-3.0</td>
</tr>
<tr>
<td>20</td>
<td>-3 + i</td>
<td>-3 - i</td>
</tr>
<tr>
<td>40</td>
<td>-3 + 3.32 i</td>
<td>-3 - 3.32 i</td>
</tr>
<tr>
<td>100</td>
<td>-3 + 6.40 i</td>
<td>-3 - 6.40 i</td>
</tr>
<tr>
<td>1000</td>
<td>-3 + 22.16 i</td>
<td>-3 - 22.16 i</td>
</tr>
</tbody>
</table>

© 2017 Pearson Education, Inc., Hoboken, NJ. All rights reserved. This material is protected under all copyright laws as they currently exist. No portion of this material may be reproduced, in any form or by any means, without permission in writing from the publisher.
Problem 2.67 Root locus plot with variation of spring constant \((k)\).

Fig. (a)
Characteristic equation:

\[ m s^2 + 12 s + 4 = 0 \]  \hspace{1cm} (1)

Roots of Eq. (1):

\[ s_{1,2} = \frac{-12 \pm \sqrt{144 - 16 m}}{2 m} \]  \hspace{1cm} (2)

Since negative and zero values of \( m \) are not possible, we vary \( m \) in the range \( 1 \leq m < \infty \). The roots given by Eq. (2) are shown in the following Table and also plotted in Fig. a.

<table>
<thead>
<tr>
<th>( m )</th>
<th>( s_1 )</th>
<th>( s_2 )</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>-0.345</td>
<td>-11.655</td>
</tr>
<tr>
<td>4</td>
<td>-0.38</td>
<td>-2.62</td>
</tr>
<tr>
<td>8</td>
<td>-0.50</td>
<td>-1.00</td>
</tr>
<tr>
<td>9</td>
<td>-0.67</td>
<td>-0.67</td>
</tr>
<tr>
<td>10</td>
<td>-0.6 + 0.2i</td>
<td>-0.6 - 0.2i</td>
</tr>
<tr>
<td>20</td>
<td>-0.3 + 0.33i</td>
<td>-0.3 - 0.33i</td>
</tr>
<tr>
<td>100</td>
<td>-0.06 + 0.19i</td>
<td>-0.06 - 0.19i</td>
</tr>
<tr>
<td>500</td>
<td>-0.012 + 0.089i</td>
<td>-0.012 - 0.089i</td>
</tr>
<tr>
<td>1000</td>
<td>-0.006 + 0.063i</td>
<td>-0.006 - 0.063i</td>
</tr>
</tbody>
</table>

\[ 2-159 \]
Problem 2.168
Root locus plot with variation of mass \((m)\).

\(\text{Fig. (a)}\)
2.169 \[ m = 20 \text{ kg}, \quad k = 4000 \text{ N/m} \]
\[
\omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{4000}{20}} = 14.1421 \text{ rad/sec}
\]
Amplitudes of successive cycles: 50, 45, 40, 35 mm
Amplitudes of successive cycles diminish by 5 mm = \(5 \times 10^{-3}\) m
System has Coulomb damping.
\[
\frac{4\mu N}{k} = 5 \times 10^{-3} \implies \mu N = \left\{ \frac{(5 \times 10^{-3})(4000)}{4} \right\} = 5 \text{ N}
\]
Damping force
Frequency of damped vibration = 14.1421 rad/sec.

2.170 \[ m = 20 \text{ kg}, \quad k = 10000 \text{ N/m}, \quad \frac{4\mu N}{k} = \frac{150 - 100}{4} \text{ mm} = 12.5 \times 10^{-3} \text{ m} \]
\[
\mu = \frac{(12.5 \times 10^{-3})(10000)}{4(20 \times 9.81)} = 0.1593
\]
Time elapsed = \(4 \tau_n = 4 \times \frac{2\pi}{\omega_n} = 8 \pi \sqrt{\frac{m}{k}} = 1.124 \text{ sec}\)

2.171 \[ m = 10 \text{ kg}, \quad k = 3000 \text{ N/m}, \quad \mu = 0.12, \quad X = 100 \text{ mm} \]
\[
\frac{4\mu N}{k} = \frac{4(0.12)(10 \times 9.81)}{3000} = 0.0157 m = 15.7 \text{ mm}
\]
As \(6\left(\frac{4\mu N}{k}\right) = 94.2 \text{ mm}, \text{ mass comes to rest at } (100 - 94.2) = 5.8 \text{ mm}\)

2.172 \[ m g = 25 \text{ N}, \quad k = 1000 \text{ N/m}, \text{ damping force = constant} \]
Mass released with \(x_0 = 10 \text{ cm}, \text{ and } \dot{x}_0 = 0\).
Static deflection of spring due to self weight of mass = \(\frac{25}{1000} = 0.025 \text{ m}\)
At \(t = 0\): \(x = 0.1 \text{ m}, \dot{x} = 0\)
\(\ddot{x}_0 = 0.1\)
\( x_1 = x_0 - 2 \frac{\mu N}{k} \), \( x_2 = x_0 - 4 \frac{\mu N}{k} \)

\( x_3 = a_0 - 6 \frac{\mu N}{k} \), \( x_4 = x_0 - 8 \frac{\mu N}{k} = 0 \)

i.e., \( x_0 = \frac{8 \mu N}{k} = 0.1 \)

Magnitude of damping force = \( \mu N = \frac{x_0 k}{8} = (0.1)(1000) \)

\[ = 12.5 \text{ N} \]

\( m = 20 \text{ kg}, \ k = 10,000 \text{ N/m}, \ \mu N = 50 \text{ N} \), \( x_0 = 0.05 \text{ m} \)

(a) Number of half cycles elapsed before mass comes to rest \((r)\) is given by:

\[ r = \left\{ \frac{x_0 - \frac{\mu N}{k}}{2 \frac{\mu N}{k}} \right\} = \frac{0.05 - \frac{50}{10000}}{2 \frac{50}{10000}} = 4.5 \]

\[ \therefore r = 5 \]

(b) Time elapsed before mass comes to rest:

\[ t_p = 2\pi \sqrt{\frac{m}{k}} = 2\pi \sqrt{\frac{20}{10000}} = 0.2810 \text{ sec} \]

Time taken = (2.5 cycles) \( t_p = 0.7025 \text{ sec} \)

(c) Final extension of spring after 5 half-cycles:

\[ x_5 = 0.05 - 5 \left( \frac{2 \frac{\mu N}{k}}{2} \right) = 0.05 - 5 \left( 2 \frac{50}{10000} \right) = 0 \]

(displacement from static equilibrium position = 0)

But static deflection \( = \frac{mg}{k} = \frac{20 \times 9.81}{10000} = 0.1962 \text{ m} \)

\[ \therefore \text{Final extension of spring} = 1.9620 \text{ cm} \]

2.174

(a) Equation of motion for angular oscillations of pendulum:

\[ I \ddot{\theta} + \mu g l \sin \theta \pm mg \mu \frac{d}{2l} \cos \theta = 0 \]

For small angles, \( \ddot{\theta} + \frac{mg l}{I} (\theta \pm \frac{\mu d}{2l}) = 0 \)

This shows that the angle of swing decreases by \( \frac{\mu d}{2l} \) in each quarter cycle.

(b) For motion from right to left:

\[ \theta(t) = A_1 \cos \omega_n t + A_2 \sin \omega_n t + \frac{\mu d}{2l} \]

where \( \omega_n = \sqrt{\frac{mg l}{I} } \)

Let \( \theta(t=0) = \theta_0 \) and \( \dot{\theta}(t=0) = 0 \). Then \( A_1 = \theta_0 - \frac{\mu d}{2l} \), \( A_2 = 0 \)

2-162
\[
\theta(t) = (\theta_0 - \frac{\mu d}{2L}) \cos \omega_n t + \frac{\mu d}{2L}
\]
For motion from left to right:
\[
\theta(t) = A_3 \cos \omega_n t + A_4 \sin \omega_n t - \frac{\mu d}{2L}
\]
At \(\omega_n t = \pi\), \(\theta = -\theta_0 + \frac{2\mu d}{2L}\), \(\dot{\theta} = 0\) from previous solution.
\[
A_3 = \theta_0 - \frac{3\mu d}{2L}, \quad A_4 = 0
\]
\[
\theta(t) = (\theta_0 - \frac{3\mu d}{2L}) \cos \omega_n t - \frac{\mu d}{2L}
\]
(c) The motion ceases when \((\theta_0 - n \frac{4\mu d}{2L}) < \frac{\mu d}{2L}\)
where \(n\) denotes the number of cycles.

2.175
\[
\chi(t) = X \sin \omega t \quad \text{(under sinusoidal force \(F_0 \sin \omega t\))}
\]
Damping force = \(\mu N\)
Total displacement per cycle = 4 \(X\)
Energy dissipated per cycle = \(\Delta W = 4 \mu N \times\)
(E1)

If \(c_{eq}\) = equivalent viscous damping constant, energy
dissipated per cycle is given by Eq. (2.98):
\[
\Delta W = \frac{\pi}{\omega} c_{eq} \omega \times^2
\]
(E2)
Equating (E1) and (E2) gives
\[
c_{eq} = \frac{4\mu N \times}{\pi \omega \times^2} = \frac{4\mu N}{\pi \omega \times}
\]
(E3)

2.176
Due to viscous damping:
\[
\delta = \ln \left(\frac{X_m}{X_{m+1}}\right) = 2\pi \frac{\mu N}{\kappa}
\]
\[
\delta_1 = \text{percent decrease in amplitude per cycle at } X_m
= 100 \left(\frac{X_m - X_{m+1}}{X_m}\right) = 100 \left(1 - \frac{X_{m+1}}{X_m}\right) = 100 \left(1 - e^{-2\pi \frac{\mu N}{\kappa} X_m}\right)
\]
Due to Coulomb damping:
\[
\delta_2 = \text{percent decrease in amplitude per cycle at } X_m
= 100 \left(\frac{X_m - X_{m+1}}{X_m}\right) = 100 \left(\frac{4\mu N}{\kappa X_m}\right)
\]
When both types of damping are present:
\[
\delta_1 + \delta_2 \bigg|_{X_m = 20 \text{ mm}} = 2 \quad ; \quad \delta_1 + \delta_2 \bigg|_{X_m = 10 \text{ mm}} = 3
\]

2-163

© 2017 Pearson Education, Inc., Hoboken, NJ. All rights reserved. This material is protected under all copyright laws as they currently exist. No portion of this material may be reproduced, in any form or by any means, without permission in writing from the publisher.
\[
\begin{align*}
100 \left(1 - e^{-2 \pi f_s}\right) + \frac{400}{0.02} \left(\frac{\mu N}{k}\right) &= 2 \\
100 \left(1 - e^{-2 \pi f_s}\right) + \frac{400}{0.01} \left(\frac{\mu N}{k}\right) &= 3
\end{align*}
\]

The solution of these equations gives
\[
50 \left(1 - e^{-2 \pi f_s}\right) = 0.5 \quad \text{and} \quad \frac{\mu N}{k} = 0.5 \times 10^{-6} \text{ m}
\]

2.177 Coulomb damping.

(a) Natural frequency \( \omega_n = \frac{2 \pi}{\tau_n} = \frac{2 \pi}{1} = 6.2832 \text{ rad/sec.} \) Reduction in amplitude in each cycle:
\[
= 4 \frac{\mu N}{k} = 4 \frac{\mu g}{k} = \frac{4 \mu g}{\omega_n^2} = 4 \mu \left(\frac{9.81}{6.2832^2}\right)
\]
\[
= 0.9940 \mu = \frac{0.5}{100} = 0.005 \text{ m}
\]
Kinetic coefficient of friction \( \mu = 0.00503 \)

(b) Number of half-cycles executed \( r \) is:
\[
r \geq \frac{(x_0 - \frac{\mu N}{k})}{(\frac{2 \mu N}{k})} =\frac{(x_0 - \frac{\mu g}{\omega_n^2})}{(\frac{2 \mu g}{\omega_n^2})}
\]
\[
\geq \frac{0.1 - 0.00503 (9.81)}{6.2832^2}
\]
\[
\geq \frac{2 (0.00503) (9.81)}{6.2832^2}
\]
\[
\geq 39.5032
\]
\[
\geq 40
\]
Thus the block stops oscillating after 20 cycles.
2.178
\[
\omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{10,000}{5}} = 44.721359 \text{ rad/s}
\]
\[
\gamma_n = \frac{2\pi}{\omega_n} = \frac{2\pi}{44.721359} = 0.140497 \text{ s}
\]
Time taken to complete 10 cycles = 10 \(\gamma_n\)
\[
= 1.40497 \text{ s}
\]

2.179
(a) \(\theta = 30^\circ\)
\[N = mg \cos \theta\]

Case 1: When \(\dot{x} = +\) and \(\ddot{x} = +\) or \(\dot{x} = -\) and \(\ddot{x} = +\):
\[m \ddot{x} = -2kx - \mu N + mg \sin \theta\]
or
\[m \ddot{x} + 2kx = -\mu mg \cos \theta + mg \sin \theta\] (E.1)

Case 2: When \(\dot{x} = +\) and \(\ddot{x} = -\) or \(\dot{x} = -\) and \(\ddot{x} = -\):
\[m \ddot{x} = -2kx + \mu N + mg \sin \theta\]
or
\[m \ddot{x} + 2kx = \mu mg \cos \theta + mg \sin \theta\] (E.2)

Eqs. (E.1) and (E.2) can be written as a single equation as:
\[m \ddot{x} + \mu mg \cos \theta \text{sgn}(\dot{x}) + 2kx + mg \sin \theta = 0\] (E.3)

(b) \(x_0 = 0.1 \text{ m}, \dot{x}_0 = 5 \text{ m/s}\)
\[
\omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{1000}{20}} = 7.071068 \text{ rad/s}
\]
Solution of Eq. (E.1):
\[x(t) = A_1 \cos \omega_n t + A_2 \sin \omega_n t - \frac{\mu mg \cos \theta}{k} + \frac{mg \sin \theta}{k}\] (E.4)

Solution of Eq. (E.2):
\[x(t) = A_3 \cos \omega_n t + A_4 \sin \omega_n t + \frac{\mu mg \cos \theta}{k} + \frac{mg \sin \theta}{k}\] (E.5)
Using the initial conditions in each half cycle, the
constants \( A_1 \) and \( A_2 \) or \( A_3 \) and \( A_4 \) are to be found.
For example, in the first half cycle, the motion
starts from left toward right with \( x_o = 0.1 \) and
\( \dot{x}_o = 5 \). These values can be used in Eq. (E.4)
to find \( A_1 \) and \( A_2 \).

\[ 2.180 \]
Friction force = \( \mu N = 0.2 \times 5 = 1 \text{ N} \). \( k = \frac{25}{0.10} = 250 \text{ N/m} \). Reduction in
amplitude in each cycle = \( \frac{4 \mu N}{k} = \frac{4 \times 1}{250} = 0.016 \text{ m} \). Number of half-cycles
executed before the motion ceases \( (r) \):

\[ r \geq \left( \frac{x_o - \mu N}{\frac{2 \mu N}{k}} \right) = \frac{0.1 - 0.004}{0.008} \text{ m} = 12 \]

Thus after 6 cycles, the mass stops at a distance of \( 0.1 - 6(0.016) = 0.004 \text{ m} \) from
the unstressed position of the spring.

\[ \omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{250(0.81)}{5}} = 22.1472 \text{ rad/sec} \]

\[ \tau_n = \frac{2 \pi}{\omega_n} = 0.2837 \text{ sec} \]

Thus total time of vibration = \( 6 \tau_n = 1.7022 \text{ sec} \).

\[ 2.181 \]
Energy dissipated in each full load cycle is given by the area
enclosed by the hysteresis loop.
The area can be found by counting the squares enclosed by
the hysteresis loop. In Fig. 2.117, the number of squares is
\( \approx 33 \). Since each square = \( \frac{100 \times 1}{1000} = 0.1 \text{ N-m} \), the
energy dissipated in a cycle is

\[ \Delta W = 33 \times 0.1 = 3.3 \text{ N-m} = \pi \kappa \beta \chi^2 \]

Since the maximum deflection = \( \chi = 4.3 \text{ mm} \), and the
slope of the force-deflection curve is

\[ k = \frac{1800 \text{ N}}{11 \text{ mm}} = 1.6364 \times 10^5 \text{ N/m} \],

the hysteresis damping constant \( \beta \) is given by

© 2017 Pearson Education, Inc., Hoboken, NJ. All rights reserved. This material is protected under all copyright laws as they currently exist. No portion of this material may be reproduced, in any form or by any means, without permission in writing from the publisher.
\[
\beta = \frac{\Delta W}{\pi \kappa \chi^2} = \frac{3.3}{\pi (1.6364 \times 10^5 \times 0.0043)^2} = 0.3472
\]

\[
\delta = \pi \beta = \text{logarithmic decrement} = \pi (0.3472) = 1.0908
\]

Equivalent viscous damping ratio = \( \xi_{\theta} = \beta/2 = 0.1736 \).

\[
\begin{align*}
X_{j} & = \frac{2+\pi \beta}{2-\pi \beta} = 1.1, \quad \beta = 0.03032 \\
C_{e_{\theta}} & = \beta \sqrt{\kappa \chi} = 0.03032 \sqrt{1 \times 2} = 0.04288 \quad \text{N-s/m} \\
\Delta W & = \pi \kappa \beta \chi^2 = \pi (2) (0.03032) \left( \frac{10}{1000} \right)^2 = 19.05 \times 10^{-6} \quad \text{N-m}
\end{align*}
\]

Logarithmic decrement = \( \delta = \frac{1}{n} \ln \left( \frac{X_{j}}{X_{j+1}} \right) \approx \pi \beta \)

For \( n \) cycles,

\[
\frac{1}{100} \ln \left( \frac{30/20}{X_{n}} \right) = 0.004055 = \pi \beta
\]

\[
\beta = 0.001291
\]

\[
\delta = \frac{1}{n} \ln \left( \frac{X_{0}}{X_{n}} \right) = \frac{1}{100} \ln \left( \frac{25}{10} \right) = 0.0091629
\]

\[
\delta = \frac{\pi \delta}{\kappa} = \frac{(0.0091629)(200)}{\pi} = 0.583327 \quad \text{N/m}
\]
(a) Equation of motion:
\[ \ddot{\theta} + \frac{g}{l} \sin \theta = 0 \]  
(1)

Linearization of \( \sin \theta \) about an arbitrary value \( \theta_0 \) using Taylor's series expansion (and retaining only up to the linear term):
\[ \sin \theta = \sin \theta_0 + \cos \theta_0 \cdot (\theta - \theta_0) + \cdots \]  
(2)

By defining \( \tilde{\theta} = \theta - \theta_0 \) so that \( \theta = \tilde{\theta} + \theta_0 \) with \( \dot{\theta} = \dot{\tilde{\theta}} \) and \( \ddot{\theta} = \ddot{\tilde{\theta}} \), we can express Eq. (1) as
\[ \ddot{\tilde{\theta}} + \frac{g}{l} \left( \sin \theta_0 + \tilde{\theta} \cos \theta_0 \right) = 0 \]  
(3)

where \( \frac{g}{l} \), \( \sin \theta_0 \) and \( \cos \theta_0 \) are constants. Eq. (3) is the desired linear equation.

(b) At the equilibrium (reference) positions indicated by
\[ \theta_e = n \pi \quad ; \quad n = 0, \pm \pi, \pm 2 \pi, \ldots \]  
(4)

\[ \sin \theta_e = \sin \theta_0 = 0 \]. Hence Eq. (3) takes the form
\[ \ddot{\tilde{\theta}} + \frac{g}{l} \cos \theta_0 \tilde{\theta} = 0 \]  
(5)

The characteristic equation corresponding to Eq. (5) is
\[ \lambda^2 + \frac{g}{l} \cos \theta_0 = 0 \]  
(6)

The roots of Eq. (6) are
\[ \lambda = \pm \sqrt{-\frac{g \cos \theta_0}{l}} \]  
(7)
For \( \theta_e = 0 \), \( s = \pm i \frac{\sqrt{g}}{\sqrt{l}} \) \hfill (8)

Both the values of \( s \) are imaginary. Hence the system is neutrally stable.

For \( \theta_e = \pi \), \( s = \pm \frac{\sqrt{g}}{\sqrt{l}} \) \hfill (9)

Here one value of \( s \) is positive and the other value of \( s \) is negative (both are real). Hence the system is unstable.

**ALTERNATIVE APPROACH:**

The potential energy of the pendulum is given by

\[ V(\theta) = V_0 - \frac{mg}{l} \cos \theta \] \hfill (10)

where \( V_0 \) is a constant. The equilibrium states, \( \theta = \theta_e \), of Eq. (10) are given by the stationary value of \( V(\theta) \):

\[ \frac{dV}{d\theta} = \frac{mg}{l} \sin \theta = 0 \] \hfill (11)

Roots of Eq. (11) give the equilibrium states as

\[ \theta_e = n \pi \quad ; \quad n = 1, \pm 1, \pm 2, \ldots \] \hfill (12)

Second derivative of \( V(\theta) \) is

\[ \frac{d^2V}{d\theta^2} = \frac{mg}{l} \cos \theta \] \hfill (13)

\[ = \begin{cases} 
\text{positive for } \theta = 0, 2\pi, 4\pi, \ldots \\
\text{negative for } \theta = \pi, 3\pi, \ldots 
\end{cases} \]

Thus the potential energy is minimum at \( \theta_e = 0, 2\pi, 4\pi, \ldots \) and maximum at \( \theta_e = \pi, 3\pi, \ldots \). Hence the pendulum is stable at \( \theta_e = 0 \) and unstable at \( \theta_e = \pi \).
(a) Equation of motion:

Mass moment of inertia of the circular disk about point O is \( J + ML^2 = J_d \). \label{eq1}

Mass moment of inertia of the rod about point O is \( J_r = \frac{1}{12} m l^2 + m \left( \frac{l}{2} \right)^2 = \frac{1}{3} m l^2 \). \label{eq2}

For small angular displacements (\( \theta \)) of the rigid bar about the pivot point O, the free body diagram is shown in Fig. a.

The equation of motion for the angular motion of the rigid bar, using Newton's second law of motion, is:

\[
\left( J_r + J_d \right) \ddot{\theta} - mg \frac{l}{2} \sin \theta - Mg L \sin \theta + c \dot{\theta} L \cos \theta + k \theta = 0 \qquad \text{(3)}
\]

Since \( \theta \) is small, \( \sin \theta \approx \theta \) and \( \cos \theta \approx 1 \). Thus Eq. (3) can be expressed as

\[
\left( J_r + J_d \right) \ddot{\theta} - \frac{mgL}{2} \theta - Mg L \theta + cL^2 + kL^2 = 0 \qquad \text{(4)}
\]

Eq. (4) can be written as

\[
J_0 \dddot{\theta} + C_1 \dot{\theta} + \kappa \theta = 0 \quad \text{(5)}
\]

where
\[ J_0 = J_r + J_d \]  
\[ C_t = C L^2 \]  
\[ k_t = -\frac{mg L}{2} - M g L + k L^2 \]

(b) The characteristic equation for the differential equation (5) is given by

\[ J_0 \dot{\lambda}^2 + C_t \dot{\lambda} + k_t = 0 \]  

whose roots are given by

\[ \lambda_{1,2} = -\frac{C_t \pm \sqrt{C_t^2 - 4 J_0 k_t}}{2 J_0} \]

It can be shown (see Section 3.11.1) that the system will be stable if \( C_t \) and \( k_t \) are positive.

In Eq. (9), \( C_t > 0 \) and \( J_0 > 0 \) while \( k_t > 0 \) only when \( k L^2 > \frac{mg L}{2} + M g L \) (i.e., when the moment due to the restoring force of the spring is larger than the moment due to the gravity force).
% Ex2_187.m
% This program will use dfuncl.m

tspan = [0: 0.05: 8];
x0 = [0.4; 0.0];
[t, x] = ode23('dfuncl', tspan, x0);
plot(t, x(:, 1));
xlabel('t');
ylabel('x(t)');

% dfuncl.m
function f = dfuncl(t, x)
    u = 0.5;
    k = 100;
    m = 5;
    f = zeros(2,1);
    f(1) = x(2);
    f(2) = -u * 9.81 * sign(x(2)) - k * x(1) / m;

% Ex2_188.m
wn = 10;
dx0 = 0;
x0 = 10;
for i = 1:101
    t(i) = 2*(i-1)/100;
    x1(i) = (x0 + (dx0 + wn*x0)*t(i))*exp(-wn*t(i));
end
x0 = 50;
for i = 1:101
    t(i) = 2*(i-1)/100;
    x2(i) = (x0 + (dx0 + wn*x0)*t(i))*exp(-wn*t(i));
end
x0 = 100;
for i = 1:101
    t(i) = 2*(i-1)/100;
    x3(i) = (x0 + (dx0 + wn*x0)*t(i))*exp(-wn*t(i));
end
x0 = 0;
dx0 = 10;
for i = 1:101
    t(i) = 2*(i-1)/100;
    x4(i) = (x0 + (dx0 + wn*x0)*t(i))*exp(-wn*t(i));
end
dx0 = 50;
for i = 1:101
    t(i) = 2*(i-1)/100;
    x5(i) = (x0 + (dx0 + wn*x0)*t(i))*exp(-wn*t(i));
end
dx0 = 100;
for i = 1:101
    t(i) = 2*(i-1)/100;
    x6(i) = (x0 + (dx0 + wn*x0)*t(i))*exp(-wn*t(i));
end
subplot(231);
plot(t,x1);
title('x0=10 dx0=0');
xlabel('t');
ylabel('x(t)');
subplot(232);
plot(t,x2);
title('x0=50 dx0=0');
xlabel('t');
ylabel('x(t)');
subplot(233);
plot(t,x3);
title('x0=100 dx0=0');
xlabel('t');
ylabel('x(t)');
subplot(234);
plot(t,x4);
title('x0=0 dx0=10');
xlabel('t');
ylabel('x(t)');
subplot(235);
plot(t,x5);
title('x0=0 dx0=50');
xlabel('t');
ylabel('x(t)');
subplot(236);
plot(t,x6);
title('x0=0 dx0=100');
xlabel('t');
ylabel('x(t)');
```matlab
% Ex2_189.m

wn = 10;
zeta = 2.0;
dx0 = 50;
x0 = 20;
c1 = ( x0*wn*( zeta + sqrt(zeta^2-1) ) + dx0 )/( 2*wn*sqrt(zeta^2-1) );
c2 = ( -x0*wn*( zeta - sqrt(zeta^2-1) ) - dx0 )/( 2*wn*sqrt(zeta^2-1) );
for i = 1:101
 t(i) = 5*(i-1)/100;
 x(i) = c1*exp( (-zeta + sqrt(zeta^2-1)) *wn*t(i) ) ... 
 + c2*exp( (-zeta - sqrt(zeta^2-1)) *wn*t(i) );
end
plot(t,x);
xlabel('t');
ylabel('x(t)');
```

2-174
Results of Ex2_190.m

--------------

>> program2
Free vibration analysis of a single degree of freedom analysis

Data:

$m$ = 4.00000000e+000
$k$ = 2.50000000e+003
$c$ = 0.00000000e+000
$x_d$ = 1.00000000e+002
$x_d0$ = -1.00000000e+001
$n$ = 50
$\Delta t$ = 1.00000000e-002

system is undamped

Results:

<table>
<thead>
<tr>
<th>$i$</th>
<th>time(i)</th>
<th>$x(i)$</th>
<th>$x\dot{}(i)$</th>
<th>$x\ddot{}(i)$</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>1.000000e-002</td>
<td>9.679228e+001</td>
<td>-6.282079e+002</td>
<td>-6.049518e+004</td>
</tr>
<tr>
<td>2</td>
<td>2.000000e-002</td>
<td>8.756649e+001</td>
<td>-1.207348e+003</td>
<td>-5.472905e+004</td>
</tr>
<tr>
<td>3</td>
<td>3.000000e-002</td>
<td>7.289623e+001</td>
<td>-1.711420e+003</td>
<td>-4.556014e+004</td>
</tr>
<tr>
<td>4</td>
<td>4.000000e-002</td>
<td>5.369364e+001</td>
<td>-2.109085e+003</td>
<td>-3.358853e+004</td>
</tr>
<tr>
<td>5</td>
<td>5.000000e-002</td>
<td>3.115264e+001</td>
<td>-2.375618e+003</td>
<td>-1.947040e+004</td>
</tr>
<tr>
<td>6</td>
<td>6.000000e-002</td>
<td>6.674722e+000</td>
<td>-2.494445e+003</td>
<td>-4.171701e+003</td>
</tr>
</tbody>
</table>

© 2017 Pearson Education, Inc., Hoboken, NJ. All rights reserved. This material is protected under all copyright laws as they currently exist. No portion of this material may be reproduced, in any form or by any means, without permission in writing from the publisher.
Results of Ex2_191.m

2.191

>> program2
Free vibration analysis
of a single degree of freedom analysis

Data:

m = 4.00000000e+000
k = 2.50000000e+003
c = 1.00000000e+002
xo = 1.00000000e+002
xd0 = -1.00000000e+001
n = 50
delt = 1.00000000e-002

System is under damped

Results:
<table>
<thead>
<tr>
<th>i</th>
<th>time(i)</th>
<th>x(i)</th>
<th>xd(i)</th>
<th>xdd(i)</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>1.000000e-02</td>
<td>9.704707e+01</td>
<td>-5.547860e+02</td>
<td>-4.678477e+04</td>
</tr>
<tr>
<td>2</td>
<td>2.000000e-02</td>
<td>8.940851e+01</td>
<td>-9.485455e+02</td>
<td>-3.216668e+04</td>
</tr>
<tr>
<td>3</td>
<td>3.000000e-02</td>
<td>7.854100e+01</td>
<td>-1.203024e+03</td>
<td>-1.901253e+04</td>
</tr>
<tr>
<td>4</td>
<td>4.000000e-02</td>
<td>6.575661e+01</td>
<td>-1.335030e+03</td>
<td>-7.722135e+03</td>
</tr>
<tr>
<td>5</td>
<td>5.000000e-02</td>
<td>5.218268e+01</td>
<td>-1.364393e+03</td>
<td>1.495649e+03</td>
</tr>
<tr>
<td>6</td>
<td>6.000000e-02</td>
<td>3.874058e+01</td>
<td>-1.312202e+03</td>
<td>8.592187e+03</td>
</tr>
</tbody>
</table>

...  

45  | 4.500000e-01   | -4.071590e-01 | 3.283434e+00  | 1.723973e+02  |
46  | 4.500000e-01   | -3.667451e-01 | 4.698554e+00  | 1.175182e+02  |
47  | 4.500000e-01   | -3.150951e-01 | 5.542443e+00  | 5.837337e+01  |
48  | 4.500000e-01   | -2.575358e-01 | 5.894760e+00  | 1.359090e+01  |
49  | 4.500000e-01   | -1.985409e-01 | 5.844858e+00  | -2.203340e+01 |
50  | 5.000000e-01   | -1.416733e-01 | 5.484453e+00  | -4.856551e+01 |

© 2017 Pearson Education, Inc., Hoboken, NJ. All rights reserved. This material is protected under all copyright laws as they currently exist. No portion of this material may be reproduced, in any form or by any means, without permission in writing from the publisher.
Results of Ex2_192.m

*****************************

>> program2

Free vibration analysis
of a single degree of freedom analysis

Data:

m = 4.00000000e+000
k = 2.50000000e+003
c = 2.00000000e+002
xo = 1.00000000e+002
xd0 = -1.00000000e+001
n = 50
delt = 1.00000000e-002

system is critically damped

Results:

<table>
<thead>
<tr>
<th>i</th>
<th>time(i)</th>
<th>x(i)</th>
<th>xd(i)</th>
<th>xdd(i)</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>1.000000e-002</td>
<td>9.727222e+001</td>
<td>-4.925915e+002</td>
<td>-3.616556e+004</td>
</tr>
<tr>
<td>2</td>
<td>2.000000e-002</td>
<td>9.085829e+001</td>
<td>-7.611960e+002</td>
<td>-1.872663e+004</td>
</tr>
<tr>
<td>3</td>
<td>3.000000e-002</td>
<td>8.252244e+001</td>
<td>-8.868682e+002</td>
<td>-7.233113e+003</td>
</tr>
<tr>
<td>4</td>
<td>4.000000e-002</td>
<td>7.342874e+001</td>
<td>-9.196986e+002</td>
<td>9.196986e+001</td>
</tr>
<tr>
<td>5</td>
<td>5.000000e-002</td>
<td>6.432033e+001</td>
<td>-8.946112e+002</td>
<td>4.530357e+003</td>
</tr>
</tbody>
</table>

© 2017 Pearson Education, Inc., Hoboken, NJ. All rights reserved. This material is protected under all copyright laws as they currently exist. No portion of this material may be reproduced, in any form or by any means, without permission in writing from the publisher.
Results of Ex2_193.m

Free vibration analysis
of a single degree of freedom analysis

Data:

\[ m = 4.00000000e+000 \]
\[ k = 2.50000000e+003 \]
\[ c = 4.00000000e+002 \]
\[ x0 = 1.00000000e+002 \]
\[ xd0 = -1.00000000e+001 \]
\[ n = 50 \]
\[ delt = 1.00000000e-002 \]

System is over damped

Results:
\[
\begin{array}{cccc}
  i & \text{time}(i) & x(i) & x_d(i) & x_{dd}(i) \\
 1 & 1.0000000e-02 & 9.754929e+01 & -3.945541e+02 & -2.157540e+04 \\
 2 & 2.0000000e-02 & 9.234636e+01 & -5.205155e+02 & -6.039927e+03 \\
 3 & 3.0000000e-02 & 8.756294e+01 & -5.463949e+02 & -8.734340e+01 \\
 4 & 4.0000000e-02 & 8.214078e+01 & -5.344391e+02 & 2.105923e+03 \\
 5 & 5.0000000e-02 & 7.691749e+01 & -5.090344e+02 & 2.830006e+03 \\
\end{array}
\]

% Ex2_194.m

2.194

This program will use dfunc2_194.m

ts0 = [0: 0.05: 12];
x0 = [0.1; 5];
[t, x] = ode23 ('dfunc2_194', ts0, x0);
plot(t, x(:, 1));
xlabel('t');
ylabel('x(t)');

% dfunc2_194.m

function f = dfunc2_194(t, x)
    u = 0.1;
k = 1000;
m = 20;
g = 9.81;
theta = 30 * pi/180;
f = zeros(2, 1);
f(1) = x(2);
f(2) = -u*g*cos(theta)*sign(x(2)) - 2*k*x(1)/m - g*sin(theta);
The equations for the natural frequencies of vibration were derived in Problem 2.35.

Operating speed of turbine is:

\[ \omega_0 = (2400) \frac{2\pi}{60} = 251.328 \text{ rad/sec} \]

Thus we need to satisfy:

\[
\begin{align*}
\omega_n \bigg|_{\text{axial}} &= \left\{ \frac{g \ell A}{W a (l-a)} \right\}^{1/2} \geq \omega_0 \quad (E_1) \\
\omega_n \bigg|_{\text{transverse}} &= \left\{ \frac{3EI \ell^3 g}{W a^3 (l-a)^3} \right\}^{1/2} \geq \omega_0 \quad (E_2) \\
\omega_n \bigg|_{\text{circumferential}} &= \left\{ \frac{G J f}{J_o} \left( \frac{1}{a} + \frac{1}{l-a} \right) \right\}^{1/2} \geq \omega_0 \quad (E_3)
\end{align*}
\]

where

\[ A = \frac{\pi d^2}{4}, \quad W = 1000 \times 9.81 = 9810 \text{ N}, \]

\[ I = \frac{\pi d^4}{64}, \quad J = \frac{\pi d^2}{32}, \quad J_o = 500 \text{ kg} \cdot \text{m}^2, \]

and

\[ E = 207 \times 10^9 \text{ N/m}^2, \quad G = 79.3 \times 10^9 \text{ N/m}^2 \text{ (for steel)}. \]

The unknowns \( d, \ell \) and \( a \) can be determined to satisfy the inequalities \((E_1), (E_2)\) and \((E_3)\) using a trial and error procedure.
From solution of problem 2.38, the requirements can be stated as:

\[
\omega_n \bigg|_{\text{pivot ends}} = \sqrt{\frac{12EI}{l^3 \left( \frac{w}{g} + m_{eff1} \right)}} \geq \omega_0 \quad (E1)
\]

where \( E = 30 \times 10^6 \text{ psi} \) and 
\[
I = \frac{\pi}{64} \left[ d^4 - (d-2t)^4 \right]
\]

\[
\omega_n \bigg|_{\text{fixed ends}} = \sqrt{\frac{48EI}{l^3 \left( \frac{w}{g} + m_{eff2} \right)}} \geq \omega_0 \quad (E2)
\]

with 
\[
m_{eff1} = 0.2357 \text{ m}, \quad m_{eff2} = 0.3714 \text{ m},
\]

\( m = \text{mass of each column} = \frac{\pi}{4} \left[ d^2 - (d-2t)^2 \right] \frac{lF}{g} \), 

\( F = 0.283 \text{ lb/in}^2 \), \( g = 386.4 \text{ in/sec}^2 \), 

\( l = \text{length of column} = 96 \text{ in.} \), 

\( W = \text{weight of floor} = 4000 \text{ lb} \), 

\( W = \text{weight of columns} = 4 \left\{ \frac{\pi}{4} \left[ d^2 - (d-2t)^2 \right] \frac{LF}{g} \right\} \quad (E3) \)

Frequency limit = \( \omega_0 = 50 \times 2\pi = 314.16 \text{ rad/sec} \).

Problem: Find \( d \) and \( t \) such that \( \omega_n \) given by \( Eg. (E3) \) is minimized while satisfying the inequalities \( (E1) \) and \( (E2) \).

This problem can be solved either by graphical optimization or by using a trial and error procedure.

\( J_0 = \frac{m l^2}{12} + \frac{m l^2}{4} + M l^2 = \frac{1}{3} m l^2 + M l^2 \quad (E1) \)

(i) Viscous damping:

\[
\omega_n = \frac{\sqrt{K_t}}{J_0} = \left( \frac{1}{\frac{1}{3} m l^2 + M l^2} \right)^{\frac{1}{2}} \quad (E2)
\]

\( (C_t)_cr = 2J_0 \omega_n = 2 \sqrt{J_0 K_t} \quad (E3) \)

For critical damping, \( Eg. (2.80) \) gives

\[
\theta(t) = \{ \theta_0 + (\dot{\theta}_0 + \omega_n \theta_0) t \} e^{-\omega_n t} \quad (E4)
\]
For \( \theta_0 = 75^\circ = 1.309 \text{ rad} \) and \( \dot{\theta}_0 = 0 \),
\[
\theta(t) = (1.309 + 1.309 \omega_n t) e^{-\omega_n t} \quad ---(E5)
\]
For \( \theta = 5^\circ = 0.08727 \text{ rad} \), Eq. (E5) becomes
\[
0.08727 = 1.309 (1 + \omega_n t) e^{-\omega_n t} \quad ---(E6)
\]
Let time to return = 2 sec. Then Eq. (E6) gives
\[
0.08727 = 1.309 (1 + 2 \omega_n) e^{-2 \omega_n} \quad ---(E7)
\]
Solve (E7) by trial and error to find \( \omega_n \). Then choose the values of \( m, M \) and \( k_t \) to get the desired value of \( \omega_n \). Find the damping constant \((c_t)_{cri}\) using Eq. (E3).

(ii) Coulomb damping:

(a) Follow the procedure of part (i) to find the value of \( \omega_n \).

(b) Derive expression for the equivalent torsional viscous damping constant \((c_t)_{eq}\) for Coulomb damping. This expression, for small amounts of damping, is
\[
(c_t)_{eq} = \left\{ \frac{4 T_d}{\pi \omega_n \Theta} \right\} \quad ---(E8)
\]
where \( T_d \) = friction (damping) torque, and \( \Theta \) = amplitude of angular oscillations.

(c) If \((c_t)_{eq}\) is to be equal to \((c_t)_{cri} = 2 \sqrt{\omega_0} k_t\), we find
\[
T_d = \frac{\pi \omega \Theta}{4} \left( 2 \sqrt{\omega_0} k_t \right) \quad ---(E9)
\]
Let \( x = \) vertical displacement of the mass (lunar excursion module), \( x_s = \) resulting deflection of each inclined leg (spring). From equivalence of potential energy, we find:

\[ k_{eq} = \text{stiffness of each leg in vertical direction} = k \cos^2 \alpha \]

Hence for the four legs, the equivalent stiffness in vertical direction is:

\[ k_{eq} = 4 k \cos^2 \alpha \]

Similarly, the equivalent damping coefficient of the four legs in vertical direction is:

\[ c_{eq} = 4 c \cos^2 \alpha \]

where \( c = \) damping constant of each leg (in axial motion). Modeling the system as a single degree of freedom system, the equation of motion is:

\[ m_{eq} \ddot{x} + c_{eq} \dot{x} + k_{eq} x = 0 \]

and the damped period of vibration is:

\[ \tau_d = \frac{2 \pi}{\omega_d} = \frac{2 \pi}{\sqrt{\frac{k_{eq}}{m_{eq}} \sqrt{1 - \left(\frac{c_{eq}^2}{4 k_{eq} m_{eq}}\right)}}} \]

Using \( m_{eq} = 2000 \text{ kg}, k_{eq} = 4 k \cos^2 \alpha, c_{eq} = 4 c \cos^2 \alpha, \) and \( \alpha = 20^\circ, \) the values of \( k \) and \( c \) can be determined (by trial and error) so as to achieve a value of \( \tau_d \) between 1 s and 2 s. Once \( k \) and \( c \) are known, the spring (helical) and damper (viscous) can be designed suitably.

Assume no damping. Neglect masses of telescoping boom and strut. Find stiffness of telescoping boom in vertical direction (see Example 2.5). Find the equivalent stiffness of telescoping boom together with the strut in vertical direction. Model the system as a single degree of freedom system with natural time period:

\[ \tau_n = \frac{2 \pi}{\omega_n} = 2 \pi \sqrt{\frac{m_{eq}}{k_{eq}}} \]

Using \( \tau_n = 1 \text{ s} \) and \( m_{eq} = \left( \frac{W_C + W_f}{g} \right) = \frac{300}{386.4}, \) determine the axial stiffness of the strut \( (k_s) \). Once \( k_s \) is known, the cross section of the strut \( (A_s) \) can be found from:

\[ k_s = \frac{A_s E_s}{\ell_s} \]

with \( E_s = 30 (10^6) \text{ psi} \) and \( \ell_s = \text{length of strut (known)} \).